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C Program name: FEM2D           Length (including blanks):3000 lines
C
C * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * *
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C * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * *
C
C . . . . . . . . . . . . . . . . . . . . . . . . . . . . .
C . This is a finite element computer program for the .
C . analysis of two-dimensional problems governed by second-order .
C . partial differential equations arising in: heat transfer, .
C . electrical engineering, fluid dynamics, and solid mechanics. .
C
C . The program uses linear and quadratic, triangular and .
C . rectangular, elements with isoparametric formulations. Meshes .
C . of only one type of element are allowed for a problem (i.e., .
C . two different types of elements cannot be used in a problem). .
C
C . Many field problems of engineering and applied science .
C . can be analyzed using this program. In particular, FEM2DV2 .
C . can be used in the finite element analysis of problems in the .
C . following fields: .
C
C . 1. Heat conduction and convection .
C . 2. Flows of viscous incompressible fluids (by penalty .
C . function formulation) .
C . 3. Plane elasticity problems .
C . 4. Plate bending problems using rectangular elements .
C . based on the classical and first-order (or Mindlin) .
C . plate theory. .
C
C . The main objective of this program is to illustrate how .
C . finite element formulations developed in Chapters 8 thru 12 .
C . can be implemented on a computer and used in the analysis of .
C . engineering problems. Modeling of large and complex problems .
C . was not an objective of the program. The program or parts of .
C . it can be modified to analyze field problems not discussed in .
C . the book. .
C
C . . . . . . . . . . . . . . . . . . . . . . .
C
C      DESCRIPTION OF SOME KEY VARIABLES USED IN THE PROGRAM
C (See Table 13.1 of the BOOK for a description of other variables)
C
C [CMAT]    Matrix of stiffnesses in elasticity and plate bending
C            problems (computed in the program from engineering
C            constants, E1, E2, G12, ANU12, etc. and thickness)
C
C {ELA}      Vector of elemental nodal accelerations
C {ELF}      Vector of element nodal source (or force) vector
C [ELK]      Element coefficient (or stiffness) matrix
C {ELU}      Vector of element nodal values of primary variables
C {ELV}      Vector of elemental nodal velocities
C {ELXY}     Vector of elemental global coordinates:
C            ELXY(I,1)=x-coordinate; ELXY(I,2)=y-coordinate
C {GLA}      Vector of global nodal accelerations
C {GLF}      Vector of global nodal source (or force) vector
C [GLK]      Global coefficient (or stiffness) matrix
C {GLU}      Vector of global nodal values of primary variables
C {GLV}      Vector of global nodal velocities
C
C NDF       Number of degrees of freedom per node:
C            NDF=1, For SINGLE VARIABLE problems
C            NDF=2, For ELASTICITY and VISCOUS FLUID FLOW
C            NDF=3, For PLATE BENDING when FSDT or CST(N)
C            elements are used
C            NDF=4, For PLATE BENDING when CST(C) element
C            is used
C
C NEQ       Total number of equations in the problem (=NNM*NDF)
C NHBW      Half band width of the global coefficient matrix, GLK
C NN        Total number of degrees of freedom per element
C
C -----
C
C      DESCRIPTION OF PARAMETERS USED TO DIMENSION THE ARRAYS
C
C MAXCNV... Maximum number of elements with convection B.C.

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C MAXELM... Maximum number of elements allowed in the program
C MAXNOD... Maximum number of nodes allowed in the program
C MAXNX.... Maximum number of allowed subdivisions DX(I) along x
C MAXNY.... Maximum number of allowed subdivisions DY(I) along y
C MAXSPV... Maximum number of specified primary variables
C MAXSSV... Maximum number of specified secondary variables
C NCMAX.... Actual column dimension of: [GLK], [GLM], {GLU}, {GLV},
C           {GLA}, and {GLF}
C
C The actual row dimension of the assembled coefficient
C matrix should be greater than or equal to the total
C number of algebraic equations in the FE model.
C
C NRMAX.... Actual row dimension of: [GLK] and [GLM]
C
C The actual column-dimension of assembled coefficient
C matrix should be greater than or equal to the half
C bandwidth for static analysis or the total number of
C equations for the dynamic analysis.
C
C NOTE: The values of NRMAX, NCMAX, MAXELM, MAXNOD, MAXCNV,
C        MAXSSV and MAXSPV in the 'PARAMETER' statement should
C        modified as required by the size of the problem.
C        When an eigenvalue problem is solved, the following
C        dimension statement should be in 'AXLBX' should be
C        modified (i.e., replace 500 with the value of NRMAX):
C
C        DIMENSION V(7500,750),VT(750,750),W(750,750),IH(750)
C
C -----
C               SUBROUTINES USED IN THE PROGRAM
C
C BOUNDARY, CONCTVITY, DATAECHO, EGNBNDRY, EGNSOLVR, ELKMFRCT, ELKMFTRI
C   EQNSOLVR, INVERSE, JACOBI, MESH2DG, MESH2DR, MATRXMLT, POSTPROC
C   QUADRTRI, SHAPERCT, SHAPETRI, TEMPORAL
C
C -----
C
C IMPLICIT REAL*8 (A-H,O-Z)
C PARAMETER (NRMAX =750,NCMAX =750,MAXELM=500,MAXNOD=500,
1      MAXSPV=500,MAXSSV=100,MAXCNV=200,MAXNX =25,MAXNY=25)
C
C DIMENSION ISPV(MAXSPV, 2), VSPV(MAXSPV), ISSV(MAXSSV, 2), VSSV(MAXSSV)
C DIMENSION IBN(MAXCNV), INOD(MAXCNV, 3), BETA(MAXSPV), TINF(MAXSSV)
C DIMENSION GLF(NRMAX), TITLE(20), IBS(3), IBL(3), GLM(NRMAX, NRMAX)
C DIMENSION GLK(NRMAX,NCMAX), GLU(NRMAX), GLV(NRMAX), GLA(NRMAX)
C DIMENSION NOD(MAXELM, 9), GLXY(MAXNOD, 2), DX(MAXNX), DY(MAXNY)
C DIMENSION EGNVAL(NRMAX), EGNVEC(NRMAX, NRMAX), IBDY(MAXSPV)
C
C COMMON/STF/ELF(27), ELK(27,27), ELM(27,27), ELXY(9,2), ELU(27),
1      ELV(27), ELA(27), A1,A2,A3,A4,A5
C COMMON/PST/A10,A1X,A1Y,A20,A2X,A2Y,A00,C0,CX,CY,F0,FX,FY,
1      C44,C55,VISCSITY,PENALTY,CMAT(3,3)
C COMMON/PNT/IPDF, IPDR, NIPF, NIPR
C COMMON/IO/IN, ITT
C COMMON/WORKSP/RWKSP
C
C * * * * * * * * * * * * * * * * * * * * * * * * * * * * *
C *
C *          P R E P R O C E S S O R      U N I T      *
C *
C * * * * * * * * * * * * * * * * * * * * * * * * * * * * *
C
C IN=5
C ITT=6
C ****
C open (in,file = ' ')
C open (itt,file = ' ')
C ****
C CALL DATAECHO(IN,ITT)
C ICONV=0
C INTIAL=0
C JVEC=1
C NSSV=0
C NFLAG=1
C
C R E A D I N      T H E      I N P U T      D A T A      H E R E

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```

C
C      READ(IN,400) TITLE
C
C      Read problem and analysis type
C
C      READ(IN,*) ITYPE,IGRAD,ITEM,NEIGN
C      IF(ITEM.EQ.0) NEIGN=0
C      IF(NEIGN.NE.0) THEN
C          IF(ITYPE.LE.3 .AND. NEIGN.GT.1) THEN
C              WRITE(ITT,991)
C              STOP
C          ELSE
C              READ(IN,*) NVALU,NVCTR
C          ENDIF
C      ENDIF
C
C      Read finite element mesh information
C
C      READ(IN,*) IELTYP,NPE,MESH,NPRNT
C      IF(ITYPE.GE.3 .AND. IELTYP.EQ.0) THEN
C          WRITE(ITT,990)
C          STOP
C      ENDIF
C      IF(NPE.LE.4) THEN
C          IEL=1
C      ELSE
C          IEL=2
C      ENDIF
C      IF(MESH.NE.1) THEN
C          READ(IN,*) NEM,NNM
C          IF(MESH.EQ.0) THEN
C
C          If mesh CANNOT be generated by the program, read the mesh data in
C          the next three statements
C
C          DO 10 N=1,NEM
10       READ(IN,*) (NOD(N,I),I=1,NPE)
            READ(IN,*) ((GLXY(I,J),J=1,2),I=1,NNM)
        ELSE
C
C          When mesh is to be generated by the program for more complicated
C          geometries, call MESH2DGeneral (which reads pertinent data there)
C
C          CALL MESH2DG(NEM,NNM,NOD,MAXELM,MAXNOD,GLXY)
        ENDIF
        ELSE
C
C          When mesh is to be generated for rectangular domains, call program
C          MESH2DRectangular, which requires the following data:
C
C          READ(IN,*) NX,NY
C          READ(IN,*) X0,(DX(I),I=1,NX)
C          READ(IN,*) Y0,(DY(I),I=1,NY)
C          CALL MESH2DR (IEL,IELTYP,NX,NY,NPE,NNM,NEM,NOD,DX,DY,X0,Y0,
*                           GLXY,MAXELM,MAXNOD,MAXNX,MAXNY)
        ENDIF
C
        IF(ITYPE.EQ.0) THEN
            NDF = 1
        ELSE
            IF(ITYPE.GE.3) THEN
                NDF = 3
            ELSE
                NDF = 2
            ENDIF
        ENDIF
        IF(ITYPE.EQ.5) NDF=4
C
        NEQ=NNM*NDF
        NN=NPE*NDF
        IF(NEIGN.EQ.0) THEN
C
C          Compute the half bandwidth of the global coefficient matrix
C
        NHBW=0
        DO 20 N=1,NEM

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DO 20 I=1,NPE
DO 20 J=1,NPE
NW= (IABS(NOD(N,I)-NOD(N,J))+1)*NDF
20 IF (NHBW.LT.NW) NHBW=NW
ELSE
  NHBW=NEQ
ENDIF
C
C Read specified primary and secondary degrees of freedom: node
C number, local degree of freedom number, and specified value.
C
READ(IN,*) NSPV
IF(NSPV.NE.0) THEN
  READ(IN,*) ((ISPV(I,J),J=1,2),I=1,NSPV)
  IF(NEIGN.EQ.0) THEN
    READ(IN,*) (VSPV(I),I=1,NSPV)
  ENDIF
ENDIF
IF(NEIGN.EQ.0) THEN
  READ(IN,*) NSSV
  IF(NSSV.NE.0) THEN
    READ(IN,*) ((ISSV(I,J),J=1,2),I=1,NSSV)
    READ(IN,*) (VSSV(I),I=1,NSSV)
  ENDIF
ENDIF
WRITE(ITT,400) TITLE
WRITE(ITT,910)
WRITE(ITT,890)
WRITE(ITT,910)
IF(ITYPE.EQ.0) THEN
C
C Heat transfer and like problems:
C Read the coefficients of the differential equation modeled _____
C A11 = A10 + A1X*X + A1Y*Y; A22 = A20 + A2X*X + A2Y*Y; A00=CONST.
C
  WRITE(ITT,410)
  READ(IN,*) A10,A1X,A1Y
  READ(IN,*) A20,A2X,A2Y
  READ(IN,*) A00
  WRITE(ITT,420) A10,A1X,A1Y,A20,A2X,A2Y,A00
  READ(IN,*) ICONV
  IF(ICONV.NE.0) THEN
    READ(IN,*) NBE
    READ(IN,*) (IBN(I),(INOD(I,J),J=1,2),BETA(I),TINF(I),I=1,NBE)
    WRITE(ITT,440) NBE
    DO 30 I=1,NBE
      30 WRITE(ITT,860) IBN(I),(INOD(I,J),J=1,2),BETA(I),TINF(I)
    ENDIF
  ELSE
    IF(ITYPE.EQ.1) THEN
C
C Viscous incompressible flows: _____
C
    WRITE(ITT,450)
    READ(IN,*) VISCSITY,PENALTY
    WRITE(ITT,460) VISCSITY,PENALTY
  ELSE
    IF(ITYPE.EQ.2) THEN
C
C Plane elasticity problems: _____
C
    READ(IN,*) LNSTRS
    WRITE(ITT,470)
    READ(IN,*) E1,E2,ANU12,G12,THKNS
    WRITE(ITT,520) THKNS,E1,E2,ANU12,G12
  C
  C Compute the material coefficient matrix, CMAT(I,J), I,J=1,2,3.
C
    ANU21=ANU12*E2/E1
    DENOM=1.0-ANU12*ANU21
    CMAT(3,3)=G12*THKNS
    IF(LNSTRS.EQ.0) THEN
C
C Plane strain (ANU23 = ANU12)
C
      WRITE(ITT,490)

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S0=(1.0-ANU12-2.0*ANU12*ANU21)
CMAT(1,1)=THKNS*E1*(1.0-ANU12)/S0
CMAT(1,2)=THKNS*E1*ANU21/S0
CMAT(2,2)=THKNS*E2*DENO/S0/(1.0+ANU12)
ELSE
C
C Plane stress
C
        WRITE(ITT,510)
        CMAT(1,1)=THKNS*E1/DENOM
        CMAT(1,2)=ANU21*CMAT(1,1)
        CMAT(2,2)=E2*CMAT(1,1)/E1
    ENDIF
ELSE
C
C Plate bending problems: _____
C
        WRITE(ITT,500)
        IF(ITYPE.EQ.3) THEN
            WRITE(ITT,505)
        ELSE
            WRITE(ITT,506)
        ENDIF
        READ(IN,*) E1,E2,ANU12,G12,G13,G23,THKNS
        WRITE(ITT,520) THKNS,E1,E2,ANU12,G12
        WRITE(ITT,530) G13,G23
        ANU21=ANU12*E2/E1
        DENOM=1.0-ANU12*ANU21
        CMAT(1,1)=(THKNS**3)*E1/DENOM/12.0D0
        CMAT(1,2)=ANU21*CMAT(1,1)
        CMAT(2,2)=E2*CMAT(1,1)/E1
        CMAT(3,3)=G12*(THKNS**3)/12.0D0
        SCF=5.0D0/6.0D0
        C44=SCF*G23*THKNS
        C55=SCF*G13*THKNS
    ENDIF
    CMAT(1,3)=0.0
    CMAT(2,3)=0.0
    CMAT(2,1)=CMAT(1,2)
    CMAT(3,1)=CMAT(1,3)
    CMAT(3,2)=CMAT(2,3)
ENDIF
C
IF(NEIGN.EQ.0) THEN
    READ(IN,*) F0,FX,FY
    WRITE(ITT,430) F0,FX,FY
ENDIF
C
IF(ITEM.NE.0) THEN
    READ(IN,*) C0,CX,CY
    IF(ITYPE.GT.1) THEN
        IF(ITYPE.EQ.2) THEN
            C0=THKNS*C0
            CX=THKNS*CX
            CY=THKNS*CY
        ELSE
            IF(NEIGN.LE.1) THEN
                C0=THKNS*C0
                CX=(THKNS**2)*C0/12.0D0
                CY=CX
            ENDIF
        ENDIF
    ENDIF
ENDIF
C
IF(NEIGN.NE.0) THEN
    WRITE(ITT,810)
    WRITE(ITT,540) C0,CX,CY
ELSE
    WRITE(ITT,820)
    WRITE(ITT,540) C0,CX,CY
C
C Read the necessary data for time-dependent problems
C
    READ(IN,*) NTIME,NSTP,INTVL,INTIAL
    IF(INTVL.LE.0) INTVL=1

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      READ(IN,*) DT,ALFA,GAMA,EPSLN
      A1=ALFA*DT
      A2=(1.0-ALFA)*DT
      WRITE(ITT,550) DT,ALFA,GAMA,NTIME,NSTP,INTVL
      IF(ITEM.EQ.1) THEN
          IF(NSSV.NE.0) THEN
              DO 40 I=1,NSSV
                  VSSV(I)=VSSV(I)*DT
          ENDIF
          IF(INTIAL.NE.0) THEN
              READ(IN,*) (GLU(I),I=1,NEQ)
          ELSE
              DO 50 I=1,NEQ
                  GLU(I)=0.0
              ENDIF
          ELSE
              DT2=DT*DT
              A3=2.0/GAMA/DT2
              A4=A3*DT
              A5=1.0/GAMA-1.0
              IF(INTIAL.NE.0) THEN
                  READ(IN,*) (GLU(I),I=1,NEQ)
                  READ(IN,*) (GLV(I),I=1,NEQ)
                  DO 60 I=1,NEQ
                      GLA(I)=0.0
              ELSE
                  DO 70 I=1,NEQ
                      GLU(I)=0.0
                      GLV(I)=0.0
                      GLA(I)=0.0
                  ENDIF
              ENDIF
          ELSE
              WRITE(ITT,830)
          ENDIF
      C
      C      *****      E N D      O F      T H E      D A T A      I N P U T      *****
      C
      IF(IELTYP.EQ.0) THEN
          WRITE(ITT,790)
      ELSE
          WRITE(ITT,800)
      ENDIF
      C
      WRITE(ITT,560) IELTYP,NPE,NDF,NEM,NNM,NEQ,NHBW
      IF(MESH.EQ.1) WRITE(ITT,570) NX,NY
      WRITE(ITT,710) NSPV
      IF(NSSV.NE.0) THEN
          WRITE(ITT,715) NSSV
          WRITE(ITT,720)
          DO 80 IB=1,NSSV
              WRITE(ITT,960)(ISSV(IB,JB),JB=1,2),VSSV(IB)
      ENDIF
      C
      IF(NPRNT.EQ.1) THEN
          WRITE(ITT,700)
          DO 100 I=1,NEM
      100     WRITE(ITT,900) I,(NOD(I,J),J=1,NPE)
      ENDIF
      C
      WRITE(ITT,910)
      WRITE(ITT,580)
      WRITE(ITT,910)
      DO 150 IM=1,NNM
      DO 110 K=1,NDF
          IBP(K)=0
      110 IBS(K)=0
      IF(NSPV.NE.0) THEN
          DO 120 JP=1,NSPV
              NODE=ISPV(JP,1)
              NDOF=ISPV(JP,2)
              IF(NODE.EQ.IM) THEN
                  IBP(NDOF)=NDOF
              ENDIF
      120 CONTINUE

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ENDIF
C
IF(NSSV.NE.0) THEN
DO 140 JS=1,NSSV
NODE=ISSV(JS,1)
NDOF=ISSV(JS,2)
IF(NODE.EQ.IM) THEN
IBS(NDOF)=NDOF
ENDIF
140 CONTINUE
ENDIF
C
IF(NDF.EQ.1) THEN
WRITE(ITT,870) IM, (GLXY(IM,J),J=1,2), (IBP(K),K=1,NDF),
*(IBS(K),K=1,NDF)
ELSE
IF(NDF.EQ.2) THEN
WRITE(ITT,920) IM, (GLXY(IM,J),J=1,2), (IBP(K),K=1,NDF),
*(IBS(K),K=1,NDF)
ELSE
IF(NDF.EQ.3) THEN
WRITE(ITT,880) IM, (GLXY(IM,J),J=1,2), (IBP(K),K=1,NDF),
*(IBS(K),K=1,NDF)
ELSE
WRITE(ITT,885) IM, (GLXY(IM,J),J=1,2), (IBP(K),K=1,NDF),
*(IBS(K),K=1,NDF)
ENDIF
ENDIF
ENDIF
150 CONTINUE
WRITE(ITT,910)
C
C Define the polynomial degree and number of integration points
C (based on the assumed variation of the coefficients AX, BX, etc.)
C
IPDR = IEL
NIPR = IPDR+IEL-1
IF(IELTYP.EQ.0) THEN
IF(ITYPE.EQ.0) THEN
IPDF = 2*IEL+1
NIPF = IPDF+IEL
ELSE
IF(ITEM.NE.0) THEN
IPDF = 2*IEL+1
NIPF = IPDF+IEL
ELSE
IPDF = IEL+1
NIPF = IPDF+1
ENDIF
ENDIF
ISTR = 1
NSTR = 1
WRITE(ITT,480) IPDF,NIPF,IPDR,NIPR,ISTR,NSTR
ELSE
IF(ITYPE.GE.4) THEN
IPDF = 4
ISTR = 2
ELSE
IPDF = IEL+1
ISTR = IEL
ENDIF
WRITE(ITT,485) IPDF,IPDR,ISTR
ENDIF
C
C * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * *
C *
C * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * *
C
C * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * *
C
C IF(ITEM.NE.0) THEN
TIME=0.0
ENDIF
C
C Counter on number of TIME steps begins here
C

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```

        NT = 0
        NCOUNT=0
170  NCOUNT=NCOUNT+1
        IF (ITEM.NE.0 .AND. NEIGN.EQ.0) THEN
            IF (NCOUNT.GE.NSTP) THEN
                F0=0.0
                FX=0.0
                FY=0.0
            ENDIF
        ENDIF
C
C      Initialize the global coefficient matrices and vectors
C
        DO 180 I=1,NEQ
            GLF(I)=0.0
        DO 180 J=1,NHBW
            IF (NEIGN.NE.0) GLM(I,J)=0.0
180  GLK(I,J)=0.0
C
C      Do-loop on the number of ELEMENTS to compute element matrices
C      and their assembly begins here
C
        DO 250 N=1,NEM
        DO 200 I=1,NPE
            NI=NOD(N,I)
            ELXY(I,1)=GLXY(NI,1)
            ELXY(I,2)=GLXY(NI,2)
            IF (NEIGN.EQ.0) THEN
                IF (ITEM.NE.0) THEN
                    LI=(NI-1)*NDF
                    L = (I-1)*NDF
                    DO 190 J=1,NDF
                        LI=LI+1
                        L=L+1
                        ELU(L)=GLU(LI)
                        IF (ITEM.EQ.2) THEN
                            ELV(L)=GLV(LI)
                            ELA(L)=GLA(LI)
                        ENDIF
                190    CONTINUE
                ENDIF
            ENDIF
200  CONTINUE
C
C      Call subroutine ELKMFTRI (for Triangular elements) or ELKMFRCT (for
C      Rectangular elements) to compute the Element [K], [M] and {F}.
C
        IF (IELTYP.EQ.0) THEN
            CALL ELKMFTRI (NEIGN,NPE,NN,ITYPE,ITEM)
        ELSE
            CALL ELKMFRCT (NEIGN,NPE,NN,ITYPE,ITEM)
        ENDIF
C
        IF (ICONV.NE.0) THEN
C
C      Add the convective terms for CONVECTION type boundary conditions
C      (exact for straight sided elements; otherwise approximate values)
C
            DO 210 M = 1,NBE
                IF (IBN(M).EQ.N) THEN
                    M1 = INOD(M,1)
                    M2 = INOD(M,2)
                    NM1 = NOD(N,M1)
                    NM2 = NOD(N,M2)
                    DL = DSQRT( (GLXY(NM2,1)-GLXY(NM1,1))**2
                    *           +(GLXY(NM2,2)-GLXY(NM1,2))**2)
                    BL = BETA(M)*DL
                    TF = TINF(M)*BL
                    IF (IEL.EQ.1) THEN
                        ELK(M1,M1)=ELK(M1,M1)+BL/3.0
                        ELK(M1,M2)=ELK(M1,M2)+BL/6.0
                        ELK(M2,M1)=ELK(M2,M1)+BL/6.0
                        ELK(M2,M2)=ELK(M2,M2)+BL/3.0
                        ELF(M1)=ELF(M1)+0.5*TF
                        ELF(M2)=ELF(M2)+0.5*TF
                    ELSE
                ENDIF
            ENDIF
        ENDIF

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        IF(NPE.GE.8) THEN
          NPEL=4
        ELSE
          NPEL=3
        ENDIF
        M3=M1+NPEL
        ELK(M1,M1)=ELK(M1,M1)+4.0*BL/30.0
        ELK(M1,M3)=ELK(M1,M3)+2.0*BL/30.0
        ELK(M1,M2)=ELK(M1,M2)-BL/30.0
        ELK(M3,M1)=ELK(M3,M1)+2.0*BL/30.0
        ELK(M3,M3)=ELK(M3,M3)+16.0*BL/30.0
        ELK(M2,M3)=ELK(M2,M3)+2.0*BL/30.0
        ELK(M2,M1)=ELK(M2,M1)-BL/30.0
        ELK(M3,M2)=ELK(M3,M2)+2.0*BL/30.0
        ELK(M2,M2)=ELK(M2,M2)+4.0*BL/30.0
        ELF(M1)=ELF(M1)+TF/6.0
        ELF(M3)=ELF(M3)+4.0*TF/6.0
        ELF(M2)=ELF(M2)+TF/6.0
      ENDIF
    ENDIF
  210  CONTINUE
ENDIF
C
C     IF(NCOUNT.EQ.1) THEN
C       IF(NPRNT.EQ.1 .OR. NPRNT.EQ.3) THEN
C         IF(N.EQ.1) THEN
C           Print element matrices and vectors (only when NPRNT=1 or NPRNT=3)
C
C           WRITE(ITT,610)
C           DO 220 I=1,NN
C             WRITE(ITT,930) (ELK(I,J),J=1,NN)
C             IF(NEIGN.EQ.0) THEN
C               WRITE(ITT,630)
C               WRITE(ITT,930) (ELF(I),I=1,NN)
C             ELSE
C               WRITE(ITT,620)
C               DO 230 I=1,NN
C                 WRITE(ITT,930) (ELM(I,J),J=1,NN)
C               ENDIF
C             ENDIF
C           ENDIF
C
C           IF(NEIGN.EQ.0) THEN
C             IF(ITEM.NE.0) THEN
C               Compute the element coefficient matrices [K-hat] and {F-hat}
C               (i.e., after time approximation) in the transient analysis:_____
C
C               CALL TEMPORAL(NCOUNT,INTIAL,ITEM,NN)
C             ENDIF
C           ENDIF
C
C           ASSEMBLE element matrices to obtain global matrices:_____
C
C           DO 240 I=1,NPE
C             NR=(NOD(N,I)-1)*NDF
C             DO 240 II=1,NDF
C               NR=NR+1
C               L=(I-1)*NDF+II
C               IF(NEIGN.EQ.0) THEN
C                 GLF(NR)=GLF(NR)+ELF(L)
C               ENDIF
C             DO 240 J=1,NPE
C               IF(NEIGN.EQ.0) THEN
C                 NCL=(NOD(N,J)-1)*NDF
C               ELSE
C                 NC=(NOD(N,J)-1)*NDF
C               ENDIF
C               DO 240 JJ=1,NDF
C                 M=(J-1)*NDF+JJ
C                 IF(NEIGN.EQ.0) THEN
C                   NC=NCL+JJ+1-NR
C                   IF(NC.GT.0) THEN
C                     GLK(NR,NC)=GLK(NR,NC)+ELK(L,M)
C                   ENDIF
C                 ENDIF
C               ENDIF
C             ENDIF
C           ENDIF
C         ENDIF
C       ENDIF
C     ENDIF

```

```

        ENDIF
    ELSE
        NC=NC+1
        GLK(NR, NC) =GLK(NR, NC) +ELK(L, M)
        GLM(NR, NC) =GLM(NR, NC) +ELM(L, M)
    ENDIF
240    CONTINUE
250    CONTINUE
C
C      Print global matrices when NPRNT > 2
C
260    IF(NCOUNT.LE.1) THEN
        IF(NPRNT.GE.2) THEN
            WRITE(ITT,640)
            DO 260 I=1,NEQ
                WRITE(ITT,930) (GLK(I,J),J=1,NHBW)
            IF(NEIGN.EQ.0) THEN
                WRITE(ITT,650)
                WRITE(ITT,930) (GLF(I),I=1,NEQ)
            ELSE
                WRITE(ITT,655)
                DO 265 I=1,NEQ
                    WRITE(ITT,930) (GLM(I,J),J=1,NEQ)
                ENDIF
            ENDIF
        ENDIF
C
C      Impose BOUNDARY CONDITIONS on primary and secondary variables
C
265    IF(NEIGN.NE.0) THEN
        CALL EGNBNDRY(GLK,GLM,IBDY,ISPV,MAXSPV,NDF,NEQ,NEQR,NSPV,NRMAX)
C
C      Call subroutine EGNSOLVR to solve for eigenvalues and eigenvectors
C      and print them as specified
C
        CALL EGNSOLVR(NEQR,GLK,GLM,EGNVAL,EGNVEC,JVEC,NROT,NRMAX)
        WRITE(ITT,660)
        WRITE(ITT,665) NROT
        IF(NVALU.GT.NEQR) NVALU=NEQR
        DO 270 I=1,NVALU
            IF(ITEM.GE.2 .AND. NEIGN.EQ.1) THEN
                VALUE = DSQRT(EGNVAL(I))
                WRITE(ITT,840) I,EGNVAL(I),VALUE
            ELSE
                WRITE(ITT,845) I,EGNVAL(I)
            ENDIF
            IF(NVCTR.NE.0) THEN
                WRITE(ITT,850)
                WRITE(ITT,930) (EGNVEC(J,I),J=1,NEQR)
            ENDIF
270    CONTINUE
        STOP
    ELSE
        CALL BOUNDARY(ISPV,ISSV,MAXSPV,MAXSSV,NDF,NCMAX,NRMAX,NEQ,
*                      NHBW,NSPV,NSSV,GLK,GLF,VSPV,VSSV,NCOUNT,INTIAL)
        IF(NCOUNT.LE.1) THEN
            IF(NPRNT.GE.2) THEN
                WRITE(ITT,650)
                WRITE(ITT,930) (GLF(I),I=1,NEQ)
            ENDIF
        ENDIF
C
C      Call subroutine EQNSOLVR to solve the system of algebraic equations
C      The solution is returned in the array GLF
C
        IRES=0
        CALL EQNSOLVR(NRMAX,NCMAX,NEQ,NHBW,GLK,GLF,IRES)
C
        IF(ITEM.NE.0) THEN
C
C          For nonzero initial conditions, GLF in the very first solution
C          is the acceleration, {A}=[MINV]({F}-[K]{U})
C
            IF(NCOUNT.EQ.1 .AND. INTIAL.NE.0) THEN
                IF(ITEM.EQ.2) THEN
                    DO 280 I=1,NEQ

```

```

        GLA(I)=GLF(I)
        WRITE(ITT,600) TIME
        WRITE(ITT,930) (GLA(I),I=1,NEQ)
        GOTO 170
    ELSE
        NT = NT + 1
        TIME=TIME+DT
    ENDIF
    ELSE
        NT = NT + 1
        TIME=TIME+DT
    ENDIF
ENDIF

C Compute the difference between solutions at two consecutive times,
C and calculate new velocities and accelerations
C
DIFF=0.0
SOLN=0.0
DO 290 I=1,NEQ
IF(ITEM.NE.0) THEN
    SOLN=SOLN+GLF(I)*GLF(I)
    DIFF=DIFF+(GLF(I)-GLU(I))*(GLF(I)-GLU(I))
ENDIF
IF(ITEM.EQ.2) THEN
    GLU(I)=A3*(GLF(I)-GLU(I))-A4*GLV(I)-A5*GLA(I)
    GLV(I)=GLV(I)+A1*GLU(I)+A2*GLA(I)
    GLA(I)=GLU(I)
ENDIF
290   GLU(I)=GLF(I)
IF(ITEM.NE.0 .AND. NT.GT.1) THEN
    NFLAG=0
    PERCNT=DSQRT(DIFF/SOLN)
    IF(PERCNT.LE.EPSLN) THEN
        WRITE(ITT,980)
        STOP
    ELSE
        INTGR=(NT/INTVL)*INTVL
        IF(INTGR.EQ.NT) NFLAG=1
    ENDIF
ENDIF
IF(NFLAG.NE.0) THEN
C Print the solution (i.e., nodal values of the primary variables)
C
    IF(ITEM.NE.0) THEN
        WRITE(ITT,590) TIME,NT
    ENDIF
    WRITE(ITT,660)
    IF(NDF.LE.3) THEN
        MDF=NDF
    ELSE
        MDF=3
        WRITE(ITT,666)
        WRITE(ITT,930) (GLU(J),J=NDF,NEQ,NDF)
    ENDIF
    IF(ITYPE.EQ.0) THEN
        WRITE(ITT,940)
    ELSE
        WRITE(ITT,970)
    ENDIF
    IF(NDF.EQ.1)WRITE(ITT,670)
    IF(NDF.EQ.2)WRITE(ITT,680)
    IF(NDF.GE.3)WRITE(ITT,690)
    IF(ITYPE.EQ.0) THEN
        WRITE(ITT,940)
    ELSE
        WRITE(ITT,970)
    ENDIF
    DO 300 I=1,NNM
        II=NDF*(I-1)+1
        JJ=II+MDF-1
        WRITE(ITT,950) I,(GLXY(I,J),J=1,2),(GLU(J),J=II,JJ)
    300   WRITE(ITT,970)
ENDIF
IF(IGRAD.NE.0) THEN

```

```

        IF(NFLAG.EQ.1) THEN
C      * * * * * * * * * * * * * * * * * * * * * * * * * * * *
C      *
C      *          P O S T P R O C E S S O R      U N I T      *
C      * * * * * * * * * * * * * * * * * * * * * * * * * * * *
C
C      IF(ITYPE.LE.1) THEN
C          WRITE(ITT,970)
C      ELSE
C          WRITE(ITT,940)
C      ENDIF
C      IF(ITYPE.LE.0) THEN
C          WRITE(ITT,730)
C          IF(IGRAD.EQ.1) THEN
C              WRITE(6,740)
C          ELSE
C              WRITE(6,750)
C          ENDIF
C      ELSE
C          IF(ITYPE.EQ.1) WRITE(ITT,760)
C          IF(ITYPE.GE.2) WRITE(ITT,770)
C          IF(ITYPE.EQ.3) WRITE(ITT,780)
C      ENDIF
C      IF(ITYPE.LE.1) THEN
C          WRITE(ITT,970)
C      ELSE
C          WRITE(ITT,940)
C      ENDIF
C
C      Compute the GRADIENT of the solution for single-variable problems
C      or STRESSES for viscous flows, plane elasticity and plate bending
C
      DO 320 N=1,NEM
      DO 310 I=1,NPE
      NI=NOD(N,I)
      ELXY(I,1)=GLXY(NI,1)
      ELXY(I,2)=GLXY(NI,2)
      LI=(NI-1)*NDF
      L=(I-1)*NDF
      DO 310 J=1,NDF
      LI=LI+1
      L=L+1
      ELU(L)=GLU(LI)
310    CONTINUE
      CALL POSTPROC(ELXY,ITYPE,IELTYP,IGRAD,NDF,NPE,THKNS,
                   ELU,ISTR,NSTR)
      IF(ITYPE.LE.1) THEN
          WRITE(ITT,970)
      ELSE
          WRITE(ITT,940)
      ENDIF
      ENDIF
      ENDIF
      ENDIF
      IF(ITEM.NE.0) THEN
          IF(NT.GE.NTIME) THEN
              STOP
          ELSE
              GOTO 170
          ENDIF
      ENDIF
      ENDIF
      STOP
C
C          F   O   R   M   A   T   S
C
400 FORMAT(20A4)
410 FORMAT (/,16X,'ANALYSIS OF A POISSON/LAPLACE EQUATION')
420 FORMAT (/,5X,'COEFFICIENTS OF THE DIFFERENTIAL EQUATION:',//,
           *, 8X,'Coefficient, A10 .....=',E12.4,,/
           *, 8X,'Coefficient, A1X .....=',E12.4,,/
           *, 8X,'Coefficient, A1Y .....=',E12.4,,/
           *, 8X,'Coefficient, A20 .....=',E12.4,,/
           *, 8X,'Coefficient, A2X .....=',E12.4,,/

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        *          8X,'Coefficient, A2Y .....=',E12.4,,/
        *          8X,'Coefficient, A00 .....=',E12.4,/)
430 FORMAT (/,5X,'CONTINUOUS SOURCE COEFFICIENTS:',//,
        *          8X,'Coefficient, F0 .....=',E12.4,,/
        *          8X,'Coefficient, FX .....=',E12.4,,/
        *          8X,'Coefficient, FY .....=',E12.4,/)
440 FORMAT (/,5X,'CONVECTIVE HEAT TRANSFER DATA:',//,
        *          8X,'Number of elements with convection, NBE .=',I4,,/
        *          8X,'Elements, their LOCAL nodes and convective',/,,
        *          8X,'heat transfer data:',/,,
        *          8X,'Ele. No.',4X,'End Nodes',8X,'Film Coeff.',6X,
        *          'T-Infinity',/)
450 FORMAT (/,16X,'A VISCOS INCOMPRESSIBLE FLOW IS ANALYZED')
460 FORMAT (/,5X,'PARAMETERS OF THE FLUID FLOW PROBLEM:',//,
        *          8X,'Viscosity of the fluid, VISCSTY .....=',E12.4,,/
        *          8X,'Penalty parameter, PENALTY .....=',E12.4,/)
470 FORMAT (/,16X,'A 2-D ELASTICITY PROBLEM IS ANALYZED')
480 FORMAT (/,5X,'NUMERICAL INTEGRATION DATA:',//,
        *          8X,'Full Integration polynomial degree, IPDF =',I4,,/
        *          8X,'Number of full integration points, NIPF =',I4,,/
        *          8X,'Reduced Integration polynomial deg.,IPDR =',I4,,/
        *          8X,'No. of reduced integration points, NIPR =',I4,,/
        *          8X,'Integ. poly. deg. for stress comp., ISTR =',I4,,/
        *          8X,'No. of integ. pts. for stress comp.,NSTR =',I4,/)
485 FORMAT (/,5X,'NUMERICAL INTEGRATION DATA:',//,
        *          8X,'Full quadrature (IPDF x IPDF) rule, IPDF =',I4,,/
        *          8X,'Reduced quadrature (IPDR x IPDR), IPDR =',I4,,/
        *          8X,'Quadrature rule used in postproc., ISTR =',I4,/)
490 FORMAT (9X,'**PLANE STRAIN assumption is selected by user**',/)
500 FORMAT (/,16X,'A PLATE BENDING PROBLEM IS ANALYZED')
505 FORMAT (16X, '*** using the shear deformation theory ***')
506 FORMAT (16X, '**** using the classical plate theory ****')
510 FORMAT (/,8X,'***PLANE STRESS assumption is selected by user**',/)
520 FORMAT (/,5X,'MATERIAL PROPERTIES OF THE SOLID ANALYZED:',//,
        *          8X,'Thickness of the body, THKNS .....=',E12.4,,/
        *          8X,'Modulus of elasticity, E1 .....=',E12.4,,/
        *          8X,'Modulus of elasticity, E2 .....=',E12.4,,/
        *          8X,'Poisson s ratio, ANU12 .....=',E12.4,,/
        *          8X,'Shear modulus, G12 .....=',E12.4)
530 FORMAT (8X,'Shear modulus, G13 .....=',E12.4,,/
        *          8X,'Shear modulus, G23 .....=',E12.4,/)
540 FORMAT (/,5X,'PARAMETERS OF THE DYNAMIC ANALYSIS:',//,
        *          8X,'Coefficient, C0 .....=',E12.4,,/
        *          8X,'Coefficient, CX .....=',E12.4,,/
        *          8X,'Coefficient, CY .....=',E12.4)
550 FORMAT (8X,'Time increment used, DT .....=',E12.4,,/
        *          8X,'Parameter, ALFA .....=',E12.4,,/
        *          8X,'Parameter, GAMA .....=',E12.4,,/
        *          8X,'Number of time steps used, NTIME .....=',I4,,/
        *          8X,'Time step at which load is removed, NSTP.=',I4,,/
        *          8X,'Time interval at which soln. is printed..=',I4,/)
560 FORMAT (/,5X,'FINITE ELEMENT MESH INFORMATION:',//,
        *          8X,'Element type: 0 = Triangle; >0 = Quad.) ..=',I4,,/
        *          8X,'Number of nodes per element, NPE .....=',I4,,/
        *          8X,'No. of primary deg. of freedom/node, NDF =',I4,,/
        *          8X,'Number of elements in the mesh, NEM .....=',I4,,/
        *          8X,'Number of nodes in the mesh, NNM .....=',I4,,/
        *          8X,'Number of equations to be solved, NEO ....=',I4,,/
        *          8X,'Half bandwidth of the matrix GLK, NHBW ..=',I4)
570 FORMAT (8X,'Mesh subdivisions, NX and NY .....=',2I4,/)
580 FORMAT (5X,'Node x-coord. y-coord. Speci. primary & seconda
        *ry variables',/,38X,'(0, unspecified; >0, specified)',/
        *          /,41X,'Primary DOF Secondary DOF')
590 FORMAT (/,5X,'*TIME* =',E12.5,5X,'Time Step Number =',I3)
600 FORMAT (/,5X,'*TIME* =',E12.5,' (Initial acceleration vector:) ',/)
610 FORMAT (/,5X,'Element coefficient matrix: ',/)
620 FORMAT (/,5X,'Element mass matrix: ',/)
630 FORMAT (/,5X,'Element source vector: ',/)
640 FORMAT (/,5X,'Global coefficient matrix (upper band) :',/)
650 FORMAT (/,5X,'Global source vector: ',/)
655 FORMAT (/,5X,'Global mass matrix (full form) :',/)
660 FORMAT (/,5X,'S O L U T I O N :',/)
665 FORMAT (/,8X,'Number of Jacobi iterations ..... NROT =',I6,/)
666 FORMAT (5X,'Nodal values of W,xy for conforming plate element:',/)
670 FORMAT (5X,'Node x-coord. y-coord. Primary DOF')
680 FORMAT (5X,'Node x-coord. y-coord. Value of u',

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*           ' Value of v')
690 FORMAT (5X,'Node      x-coord.      y-coord.      deflec. w',
*           '      x-rotation    y-rotation')
700 FORMAT (/,5X,'Connectivity Matrix, [NOD]',/)
710 FORMAT (8X,'No. of specified PRIMARY variables, NSPV =',I4)
715 FORMAT (8X,'No. of speci. SECONDARY variables, NSSV =',I4,/)
720 FORMAT (6X,'Node DOF      Value',/)
730 FORMAT (4X,'The orientation of gradient vector is measured from
1the positive x-axis',/)
740 FORMAT (4X,'x-coord.      y-coord. -a11(du/dx) -a22(du/dy)',,
1      3X,'Flux Mngtd Orientation')
750 FORMAT (4X,'x-coord.      y-coord. a22(du/dy) -a11(du/dx)',,
1      3X,'Flux Mngtd Orientation')
760 FORMAT (5X,'x-coord.      y-coord. sigma-x      sigma-y',
*'      sigma-xy      pressure')
770 FORMAT (5X,'x-coord.      y-coord. sigma-x      sigma-y',
*'      sigma-xy')
780 FORMAT (5X,'                                sigma-xz      sigma-yz')
790 FORMAT (/,8X,'*** A mesh of TRIANGLES is chosen by user ***')
800 FORMAT (/,8X,'*** A mesh of QUADRILATERALS is chosen by user ***')
810 FORMAT (/,8X,'***** An EIGENVALUE PROBLEM is analyzed *****')
820 FORMAT (/,8X,'***** A TRANSIENT PROBLEM is analyzed *****')
830 FORMAT (/,8X,'***** A STEADY-STATE PROBLEM is analyzed *****')
840 FORMAT (/,3X,'Eigenvalue(' ,I3,') =' ,E15.6,3X,'Frequency =' ,E13.5)
845 FORMAT (8X,'E I G E N V A L U E (' ,I3,') =' ,E15.6)
850 FORMAT (/,8X,'E I G E N V E C T O R :',/)
860 FORMAT (8X,I5,5X,2I5,6X,E13.5,5X,E13.5)
870 FORMAT (5X,I3,2E12.4,8X,I9,9X,I5)
880 FORMAT (5X,I3,2E12.4,7X,3I4,2X,3I4)
885 FORMAT (5X,I3,2E12.4,5X,4I4,2X,4I4)
890 FORMAT (12X,'OUTPUT from program *** FEM2D *** by J. N. REDDY')
900 FORMAT (10X,10I5)
910 FORMAT (2X,70(' '),/)
920 FORMAT (5X,I3,2E12.4,8X,2I5,4X,2I5)
930 FORMAT (8X,5E14.5)
940 FORMAT (2X,65(' '),/)
950 FORMAT (5X,I3,5E14.5)
960 FORMAT (5X,I5,I4,E14.5)
970 FORMAT (2X,77(' '),/)
980 FORMAT (/,3X,'*** THE SOLUTION HAS REACHED A STEADY STATE ***')
990 FORMAT (/,3X,'**TRIANGULAR ELEMENTS ARE NOT ALLOWED FOR PLATES**')
991 FORMAT (/,3X,'*STABILITY ANALYSIS IS ONLY FOR BENDING OF PLATES*',
*          '/3X,'**** according to the classical plate theory ****')
END

```

SUBROUTINE EGNSOLVR(N,A,B,XX,X,NEGN,NR,MXNEQ)

C Subroutine to solve the EIGENVALUE PROBLEM:

C [A] {X} = Lambda. [B] {X}

C The program can be used only for positive-definite [B] matrix
C The dimensions of V, VT, W, and IH should be equal to MXNEQ

IMPLICIT REAL*8 (A-H,O-Z)
DIMENSION A(MXNEQ,MXNEQ), B(MXNEQ,MXNEQ), XX(MXNEQ), X(MXNEQ,MXNEQ)
DIMENSION V(750,750), VT(750,750), W(750,750), IH(750)

C Call JACOBI to diagonalize [B]

C CALL JACOBI (N,B,NEGN,NR,V,XX,IH,MXNEQ)

C Make diagonalized [B] symmetric

C DO 10 I=1,N
DO 10 J=1,N

10 B(J,I)=B(I,J)

C Check (to make sure) that [B] is positive-definite

C DO 30 I=1,N
IF (B(I,I))20,30,30

20 WRITE(6,80)

```

STOP
30 CONTINUE
C
C      The eigenvectors of [B] are stored in array V(I,J)
C      Form the transpose of [V] as [VT]
C
C      DO 40 I=1,N
C      DO 40 J=1,N
40  VT(I,J)=V(J,I)
C
C      Find the product [F]=[VT] [A] [V] and store in [A] to save storage
C
C      CALL MATRXMLT (MXNEQ,N,VT,A,W)
C      CALL MATRXMLT (MXNEQ,N,W,V,A)
C
C      Get [GI] from diagonalized [B], but store it in [B]
C
C      DO 50 I=1,N
50  B(I,I)=1.0/DSQRT(B(I,I))
C
C      Find the product [Q]=[GI] [F] [GI]=[B] [A] [B] and store in [A]
C
C      CALL MATRXMLT (MXNEQ,N,B,A,W)
C      CALL MATRXMLT (MXNEQ,N,W,B,A)
C
C      We now have the form [Q]{Z}=Lamda{Z}. Diagonalize [Q] to obtain
C      the eigenvalues by calling JACOBI.
C
C      CALL JACOBI (N,A,NEGN,NR,VT,XX,IH,MXNEQ)
C
C      The eigenvalues are returned as diag [A].
C
C      DO 60 J=1,N
60  XX(J)=A(J,J)
C
C      The eigenvectors are computed from the relation,
C          {X}=[V] [GI] {Z}=[V] [B] [VT]
C      since {Z} is stored in [VT].
C
C      CALL MATRXMLT (MXNEQ,N,V,B,W)
C      CALL MATRXMLT (MXNEQ,N,W,VT,X)
C
80 FORMAT(/'*** Matrix [GLM] is NOT positive-definite ***')
RETURN
END

SUBROUTINE BOUNDARY(ISPV,ISSV,MAXSPV,MAXSSV,NDF,NCMAX,NRMAX,NEQ,
*                      NHBW,NSPV,NSSV,S,SL,VSPV,VSSV,NCOUNT,INTIAL)
C
C
C      Called in MAIN to implement specified values of the primary and
C      secondary variables by modifying the coefficient matrix [S] and
C      (banded and symmetric) and the right-hand side vector {SL}.
C
IMPLICIT REAL*8 (A-H,O-Z)
DIMENSION S(NRMAX,NCMAX), SL(NRMAX), ISPV(MAXSPV,2), VSPV(MAXSPV),
*                      ISSV(MAXSSV,2), VSSV(MAXSSV)
COMMON/IO/IN, ITT
C
IF(NSSV.NE.0) THEN
    IF(INTIAL.EQ.0 .OR. NCOUNT.NE. 1) THEN
C
C      Implement specified values of the SECONDARY VARIABLES: _____
C
        DO 10 I=1,NSSV
        II=(ISSV(I,1)-1)*NDF+ISSV(I,2)
10       SL(II)=SL(II)+VSSV(I)
        ENDIF
    ENDIF
C
C      Implement specified values of the PRIMARY VARIABLES: _____
C
IF(NSPV.NE.0) THEN
    DO 50 NB=1,NSPV

```

```

IE=(ISPV(NB, 1) - 1) *NDF+ISPV(NB, 2)
VALUE=VSPV(NB)
IT=NHBW-1
I=IE-NHBW
DO 30 II=1,IT
I=I+1
IF(I.GE.1) THEN
  J=IE-I+1
  SL(I)=SL(I)-S(I,J)*VALUE
  S(I,J)=0.0
ENDIF
30  CONTINUE
S(IE,1)=1.0
SL(IE)=VALUE
I=IE
DO 40 II=2,NHBW
I=I+1
IF(I.LE.NEQ) THEN
  SL(I)=SL(I)-S(IE,II)*VALUE
  S(IE,II)=0.0
ENDIF
40  CONTINUE
50  CONTINUE
ENDIF
RETURN
END

```

SUBROUTINE CONCTVTY(NELEM, NODES, MAXELM, MAXNOD, GLXY)

```

C
C Generates nodal connectivity array for a specified type of mesh
C
C NEL1      = First element in the row of elements
C NELL      = Last element in the row
C IELINC    = Increment from element to the next in the row
C NODINC    = Node increment from one element to the next
C NPE       = Number of nodes per element
C NODE(I)   = Global node numbers corresponding to the local nodes
C             of the first element in the row
C

```

```

IMPLICIT REAL*8(A-H,O-Z)
DIMENSION NODES(MAXELM,9), GLXY(MAXNOD,2), NODE(9)
C
C Read element data
C
READ(5,*) NRECEL
DO 30 IREC=1,NRECEL
READ(5,*) NEL1,NELL,IELINC,NODINC,NPE, (NODE(I),I=1,NPE)
IF (IELINC.LE.0) IELINC=1
IF (NODINC.LE.0) NODINC=1
IF (NELL.LE.NEL1) NELL=NEL1
IF (NELL.GT.NELEM) THEN
  WRITE(6,60)
  STOP
ELSE
  NINC=-1
  DO 20 N=NEL1,NELL,IELINC
  NINC=NINC+1
  DO 10 M=1,NPE
10    NODES(N,M)=NODE(M)+NINC*NODINC
20    CONTINUE
ENDIF
30 CONTINUE
C
DO 50 N=1,NELEM
SUMX=0.0
SUMY=0.0
NEN=NPE
IF (NEN.NE.4) THEN
  DO 40 M=5,NEN
  MM=NODES(N,M)
  IF (M.NE.9 .OR. M.NE.6) THEN
    M4=NODES(N,M-4)
    M3=NODES(N,M-3)

```

```

        IF (M.EQ.8) M3=NODES(N,1)
        IF(GLXY(MM,1).EQ.1.E20)
*          GLXY(MM,1)=0.5*(GLXY(M4,1)+GLXY(M3,1))
        IF(GLXY(MM,2).EQ.1.E20)
*          GLXY(MM,2)=0.5*(GLXY(M4,2)+GLXY(M3,2))
        IF(NEN.NE.8) THEN
          SUMX=SUMX+GLXY(M4,1)
          SUMY=SUMY+GLXY(M4,2)
        ENDIF
      ELSE
        IF(GLXY(MM,1).EQ.1.E20) GLXY(MM,1)=0.25*SUMX
        IF(GLXY(MM,2).EQ.1.E20) GLXY(MM,2)=0.25*SUMY
      ENDIF
    CONTINUE
  ENDIF
CONTINUE
FORMAT(//,'MSG from CNCTVT: Element number exceeds maximum value')
RETURN
END

```

SUBROUTINE EGNBNDRY(A,D,IBDY,ISPV,MXPV,NDF,NEQ,NEQR,NSPV,NRM)

C
C
C Imposes specified homogeneous boundary conditions on the primary
C variables by eliminating rows and columns corresponding to the
C specified degrees of freedom

```

IMPLICIT REAL*8 (A-H,O-Z)
DIMENSION A(NRM,NRM),D(NRM,NRM),ISPV(MXPV,2),IBDY(MXPV)

DO 10 I=1,NSPV
10 IBDY(I)=(ISPV(I,1)-1)*NDF+ISPV(I,2)
DO 30 I=1,NSPV
IMAX=IBDY(I)
DO 20 J=I,NSPV
IF (IBDY(J).GE.IMAX) THEN
  IMAX=IBDY(J)
  IKEPT=J
ENDIF
20 CONTINUE
IBDY(IKEPT)=IBDY(I)
IBDY(I)=IMAX
30 CONTINUE
NEQR = NEQ
DO 80 I=1,NSPV
IB=IBDY(I)
IF (IB .LT. NEQR) THEN
  NEQR1=NEQR-1
  DO 60 II=IB,NEQR1
  DO 40 JJ=1,NEQR
    D(II,JJ)=D(II+1,JJ)
40      A(II,JJ)=A(II+1,JJ)
  DO 50 JJ=1,NEQR
    D(JJ,II)=D(JJ,II+1)
50      A(JJ,II)=A(JJ,II+1)
60 CONTINUE
ENDIF
NEQR=NEQR-1
80 CONTINUE
RETURN
END

```

SUBROUTINE INVERSE(A,B)
IMPLICIT REAL*8 (A-H,O-Z)

C
C
C Called in SHAPERCT to compute the inverse of a 3x3 matrix, [A].
C The inverse is stored in matrix [B]

```

DIMENSION A(3,3), B(3,3)
G(Z1,Z2,Z3,Z4) = Z1*Z2 - Z3*Z4

```

```

F(Z1,Z2,Z3,Z4) = G(Z1,Z2,Z3,Z4) / DET
C1 = G(A(2,2),A(3,3),A(2,3),A(3,2))
C2 = G(A(2,3),A(3,1),A(2,1),A(3,3))
C3 = G(A(2,1),A(3,2),A(2,2),A(3,1))
DET = A(1,1)*C1 + A(1,2)*C2 + A(1,3)*C3
B(1,1) = F(A(2,2),A(3,3),A(3,2),A(2,3))
B(1,2) = -F(A(1,2),A(3,3),A(1,3),A(3,2))
B(1,3) = F(A(1,2),A(2,3),A(1,3),A(2,2))
B(2,1) = -F(A(2,1),A(3,3),A(2,3),A(3,1))
B(2,2) = F(A(1,1),A(3,3),A(3,1),A(1,3))
B(2,3) = -F(A(1,1),A(2,3),A(1,3),A(2,1))
B(3,1) = F(A(2,1),A(3,2),A(3,1),A(2,2))
B(3,2) = -F(A(1,1),A(3,2),A(1,2),A(3,1))
B(3,3) = F(A(1,1),A(2,2),A(2,1),A(1,2))
RETURN
END

```

SUBROUTINE DATAECHO(IN,IT)

```

C
DIMENSION AA(20)
WRITE(IT,40)
10 CONTINUE
READ(IN,30,END=20) AA
WRITE(IT,30) AA
GO TO 10
20 CONTINUE
REWIND(IN)
WRITE(IT,50)
RETURN
30 FORMAT(20A4)
40 FORMAT(5X,'*** ECHO OF THE INPUT DATA STARTS ***',/)
50 FORMAT(5X,'*** ECHO OF THE INPUT DATA ENDS ***',/)
END

```

SUBROUTINE ELKMFRC(NEIGN,NPE,NN,ITYPE,ITEM)

```

C
C
C Called in MAIN to compute element matrices based on linear and
C quadratic ReCTangular elements and isoparametric formulation for
C for all classes of problems of the book. Reduced integration is
C used on certain terms of viscous flow and plate bending problems.
C

```

```

IMPLICIT REAL*8 (A-H,O-Z)
COMMON/STF/ELF(27),ELK(27,27),ELM(27,27),ELXY(9,2),ELU(27),
1      ELV(27),ELA(27),A1,A2,A3,A4,A5
COMMON/PST/A10,A1X,A1Y,A20,A2X,A2Y,A00,C0,CX,CY,F0,FX,FY,
1      C44,C55,VISCSITY,PENALTY,CMAT(3,3)
COMMON/SHP/SF(9),GDSF(2,9),SFH(16),GDSFH(2,16),GDDSFH(3,16)
COMMON/PNT/IPDF,IPDR,NIPF,NIPR
DIMENSION GAUSPT(5,5),GAUSWT(5,5)
COMMON/IO/IN,ITT

```

```

C
DATA GAUSPT/5*0.0D0, -0.57735027D0, 0.57735027D0, 3*0.0D0,
2   -0.77459667D0, 0.0D0, 0.77459667D0, 2*0.0D0, -0.86113631D0,
3   -0.33998104D0, 0.33998104D0, 0.86113631D0, 0.0D0, -0.90617984D0,
4   -0.53846931D0, 0.0D0, 0.53846931D0, 0.90617984D0/

```

```

C
DATA GAUSWT/2.0D0, 4*0.0D0, 2*1.0D0, 3*0.0D0, 0.55555555D0,
2   0.88888888D0, 0.55555555D0, 2*0.0D0, 0.34785485D0,
3   2*0.65214515D0, 0.34785485D0, 0.0D0, 0.23692688D0,
4   0.47862867D0, 0.56888888D0, 0.47862867D0, 0.23692688D0/

```

```

C
NDF = NN/NPE
IF (ITYPE.LE.3) THEN
  NET=NPE
ELSE
  NET=NN
ENDIF

```

```

C
Initialize the arrays
C
DO 120 I = 1,NN
IF (NEIGN.EQ.0) THEN

```

```

    ELF(I) = 0.0
ENDIF
DO 120 J = 1,NN
IF (ITEM.NE.0) THEN
    ELM(I,J)= 0.0
ENDIF
120 ELK(I,J)= 0.0
C
C      Do-loops on numerical (Gauss) integration begin here. Subroutine
C      SHAPERCT (SHAPE functions for ReCTangular elements) is called here
C
DO 200 NI = 1,IPDF
DO 200 NJ = 1,IPDF
XI = GAUSPT(NI,IPDF)
ETA = GAUSPT(NJ,IPDF)
CALL SHAPERCT (NPE,XI,ETA,DET,ELXY,NDF,ITYPE)
CNST = DET*GAUSWT(NI,IPDF)*GAUSWT(NJ,IPDF)
X=0.0
Y=0.0
DO 140 I=1,NPE
X=X+ELXY(I,1)*SF(I)
140 Y=Y+ELXY(I,2)*SF(I)
C
IF (NEIGN.EQ.0) THEN
    SOURCE=F0+FX*X+FY*Y
ENDIF
IF (ITEM.NE.0) THEN
    IF (ITYPE.LE.2) THEN
        CT=C0+CX*X+CY*Y
    ENDIF
ENDIF
IF (ITYPE.LE.0) THEN
    A11=A10+A1X*X+A1Y*Y
    A22=A20+A2X*X+A2Y*Y
ENDIF
C
II=1
DO 180 I=1,NET
JJ=1
DO 160 J=1,NET
IF (ITYPE.LE.3) THEN
    S00=SF(I)*SF(J)*CNST
    S11=GDSF(1,I)*GDSF(1,J)*CNST
    S22=GDSF(2,I)*GDSF(2,J)*CNST
    S12=GDSF(1,I)*GDSF(2,J)*CNST
    S21=GDSF(2,I)*GDSF(1,J)*CNST
ENDIF
IF (ITYPE.EQ.0) THEN
C      Heat transfer and like problems (i.e. single DOF problems): _____
C
ELK(I,J) = ELK(I,J) + A11*S11 + A22*S22 + A00*S00
IF (ITEM.NE.0) THEN
    ELM(I,J) = ELM(I,J) + CT*S00
ENDIF
ELSE
    IF (ITYPE.EQ.1) THEN
C      Viscous incompressible fluids:
C      Compute coefficients associated with viscous terms (full integ.) --
C
        ELK(II,JJ) = ELK(II,JJ) + VISCSITY*(2.0*S11 + S22)
        ELK(II+1,JJ) = ELK(II+1,JJ) + VISCSITY*S12
        ELK(II,JJ+1) = ELK(II,JJ+1) + VISCSITY*S21
        ELK(II+1,JJ+1) = ELK(II+1,JJ+1) + VISCSITY*(S11 + 2.0*S22)
        IF (ITEM.NE.0) THEN
            ELM(II,JJ) = ELM(II,JJ) + CT*S00
            ELM(II+1,JJ+1) = ELM(II+1,JJ+1) + CT*S00
        ENDIF
    ELSE
        IF (ITYPE.EQ.2) THEN
C      Plane elasticity problems: _____
C
            ELK(II,JJ) = ELK(II,JJ) + CMAT(1,1)*S11+CMAT(3,3)*S22
            ELK(II,JJ+1) = ELK(II,JJ+1) + CMAT(1,2)*S12+CMAT(3,3)*S21
        ENDIF
    ENDIF
ENDIF

```

```

ELK(II+1,JJ) = ELK(II+1,JJ) + CMAT(1,2)*S21+CMAT(3,3)*S12
ELK(II+1,JJ+1)=ELK(II+1,JJ+1)+CMAT(3,3)*S11+CMAT(2,2)*S22
IF(ITEM.NE.0) THEN
    ELM(II,JJ) = ELM(II,JJ) + CT*S00
    ELM(II+1,JJ+1)= ELM(II+1,JJ+1) + CT*S00
ENDIF
ELSE
    IF(ITYPE.GE.4) THEN
C
C Classical plate theory: _____
C
    BM1=CMAT(1,1)*GDDSFH(1,J)+CMAT(1,2)*GDDSFH(2,J)
    BM2=CMAT(1,2)*GDDSFH(1,J)+CMAT(2,2)*GDDSFH(2,J)
    BM6=2.0*CMAT(3,3)*GDDSFH(3,J)
    ELK(I,J)=ELK(I,J)+CNST*(GDDSFH(1,I)*BM1
*                                +GDDSFH(2,I)*BM2+2.0*GDDSFH(3,I)*BM6)
    IF(ITEM.NE.0) THEN
        S00=SFH(I)*SFH(J)*CNST
        SXX=GDSFH(1,I)*GDSFH(1,J)*CNST
        SYY=GDSFH(2,I)*GDSFH(2,J)*CNST
        IF(NEIGN.LE.1) THEN
            ELM(I,J)=ELM(I,J) + C0*S00+CX*SXX+CY*SYY
        ELSE
            SXY=GDSFH(1,I)*GDSFH(2,J)*CNST
            SYX=GDSFH(2,I)*GDSFH(1,J)*CNST
            ELM(I,J)=ELM(I,J) + C0*SXX + CX*SYY
*                                + CY*(SXY + SYX)
        ENDIF
    ENDIF
    ELSE
C
C Shear deformable plate theory: _____
C
        ELK(II+1,JJ+1)= ELK(II+1,JJ+1) +
*                                CMAT(1,1)*S11+CMAT(3,3)*S22
        ELK(II+1,JJ+2)= ELK(II+1,JJ+2) +
*                                CMAT(1,2)*S12+CMAT(3,3)*S21
        ELK(II+2,JJ+1)= ELK(II+2,JJ+1) +
*                                CMAT(3,3)*S12+CMAT(1,2)*S21
        ELK(II+2,JJ+2)= ELK(II+2,JJ+2) +
*                                CMAT(3,3)*S11+CMAT(2,2)*S22
        IF(ITEM.NE.0) THEN
            IF(NEIGN.LE.1) THEN
                ELM(II,JJ) = ELM(II,JJ) + C0*S00
                ELM(II+1,JJ+1)= ELM(II+1,JJ+1) + CX*S00
                ELM(II+2,JJ+2)= ELM(II+2,JJ+2) + CY*S00
            ELSE
                ELM(II,JJ) = ELM(II,JJ) + C0*S11+CX*S22
*                                +CY*(S12+S21)
            ENDIF
        ENDIF
        ENDIF
        ENDIF
    ENDIF
160 JJ = NDF*I+1
    IF(NEIGN.EQ.0) THEN
C
C Source of the form fx = F0 + FX*X + FY*Y is assumed
C
        IF(ITYPE.LE.3) THEN
            L=(I-1)*NDF+1
            ELF(L) = ELF(L)+CNST*SF(I)*SOURCE
        ELSE
            ELF(I) = ELF(I)+CNST*SFH(I)*SOURCE
        ENDIF
    ENDIF
180 II = NDF*I+1
200 CONTINUE
C
    IF(ITYPE.EQ.1 .OR. ITYPE.EQ.3) THEN
C
C Use reduced integration to evaluate coefficients associated with
C penalty terms for flows and transverse shear terms for plates.
C
        DO 280 NI=1,IPDR

```

```

DO 280 NJ=1, IPDR
XI = GAUSPT(NI, IPDR)
ETA = GAUSPT(NJ, IPDR)
CALL SHAPERCT (NPE, XI, ETA, DET, ELXY, NDF, ITYPE)
CNST=DET*GAUSWT(NI, IPDR) *GAUSWT(NJ, IPDR)

C
II=1
DO 260 I=1,NPE
JJ = 1
DO 240 J=1,NPE
S11=GDSF(1,I)*GDSF(1,J)*CNST
S22=GDSF(2,I)*GDSF(2,J)*CNST
S12=GDSF(1,I)*GDSF(2,J)*CNST
S21=GDSF(2,I)*GDSF(1,J)*CNST
IF (ITYPE.EQ.1) THEN
C
C Viscous incompressible fluids (penalty terms): _____
C
ELK(II,JJ) = ELK(II,JJ) + PENALTY*S11
ELK(II+1,JJ) = ELK(II+1,JJ) + PENALTY*S21
ELK(II,JJ+1) = ELK(II,JJ+1) + PENALTY*S12
ELK(II+1,JJ+1)= ELK(II+1,JJ+1) + PENALTY*S22
ELSE
C
C Shear deformable plates (transverse shear terms): _____
C
S00=SF(I)*SF(J)*CNST
S10 = GDSF(1,I)*SF(J)*CNST
S01 = SF(I)*GDSF(1,J)*CNST
S20 = GDSF(2,I)*SF(J)*CNST
S02 = SF(I)*GDSF(2,J)*CNST
ELK(II,JJ) = ELK(II,JJ) + C55*S11+C44*S22
ELK(II,JJ+1) = ELK(II,JJ+1) + C55*S10
ELK(II+1,JJ) = ELK(II+1,JJ) + C55*S01
ELK(II,JJ+2) = ELK(II,JJ+2) + C44*S20
ELK(II+2,JJ) = ELK(II+2,JJ) + C44*S02
ELK(II+1,JJ+1)= ELK(II+1,JJ+1) + C55*S00
ELK(II+2,JJ+2)= ELK(II+2,JJ+2) + C44*S00
ENDIF
240 JJ=NDF*J+1
260 II=NDF*I+1
280 CONTINUE
ENDIF
RETURN
END

```

SUBROUTINE ELKMFTRI (NEIGN, NPE, NN, ITYPE, ITEM)

C
C Called in MAIN to compute element matrices based on linear and
C quadratic TRIangular elements and isoparametric formulation for
C for all classes of problems of the book. Reduced integration is
C used on certain terms of viscous flow and plate bending problems.
C

```

IMPLICIT REAL*8 (A-H,O-Z)
COMMON/STF/ELF(27),ELK(27,27),ELM(27,27),ELXY(9,2),ELU(27),
1           ELV(27),ELA(27),A1,A2,A3,A4,A5
COMMON/PST/A10,A1X,A1Y,A20,A2X,A2Y,A00,C0,CX,CY,F0,FX,FY,
1           C44,C55,VISCSITY,PENALTY,CMAT(3,3)
COMMON/QUAD/AL1(7,5),AL2(7,5),AL3(7,5),ALWT(7,5)
COMMON/PNT/IPDF,IPDR,NIPF,NIPR
COMMON/SHP/SF(9),GDSF(2,9),SFH(16),GDSFH(2,16),GDDSFH(3,16)
COMMON/IO/IN,ITT
C
NDF = NN/NPE
C
Call subroutine QUADRature for TRIangle to compute arrays of
C integration points and weights for the given NIPF and IPDF
C
CALL QUADRTRI (NIPF,IPDF)
C
C Initialize the arrays
C
DO 120 I = 1,NN

```

```

IF (NEIGN.EQ.0) THEN
  ELF(I) = 0.0
ENDIF
DO 120 J = 1,NN
  IF (ITEM.NE.0) THEN
    ELM(I,J)= 0.0
  ENDIF
120 ELK(I,J)= 0.0
C
C   Do-loop on the numerical integration begins here
C
  DO 200 NI = 1,NIPF
    AC1 = AL1(NI,IPDF)
    AC2 = AL2(NI,IPDF)
    AC3 = AL3(NI,IPDF)
    CALL SHAPETRI(NPE,AC1,AC2,AC3,DET,ELXY)
    CNST = 0.50D0*DET*ALWT(NI,IPDF)
    X=0.0
    Y=0.0
    DO 140 I=1,NPE
      X=X+ELXY(I,1)*SF(I)
    140 Y=Y+ELXY(I,2)*SF(I)
C
    IF (NEIGN.EQ.0) THEN
      SOURCE=F0+FX*X+FY*Y
    ENDIF
    IF (ITEM.NE.0) THEN
      CT =C0+CX*X+CY*Y
    ENDIF
    IF (ITYPE.LE.0) THEN
      A11=A10+A1X*X+A1Y*Y
      A22=A20+A2X*X+A2Y*Y
    ENDIF
C
    II=1
    DO 180 I=1,NPE
      JJ=1
      DO 160 J=1,NPE
        S00=SF(I)*SF(J)*CNST
        S11=GDSF(1,I)*GDSF(1,J)*CNST
        S22=GDSF(2,I)*GDSF(2,J)*CNST
        S12=GDSF(1,I)*GDSF(2,J)*CNST
        S21=GDSF(2,I)*GDSF(1,J)*CNST
        IF (ITYPE.EQ.0) THEN
C
C   Heat transfer and like problems (i.e. single DOF problems):_____
C
          ELK(I,J) = ELK(I,J) + A11*S11 + A22*S22 + A00*S00
          IF (ITEM.NE.0) THEN
            ELM(I,J) = ELM(I,J) + CT*S00
          ENDIF
        ELSE
          IF (ITYPE.EQ.1) THEN
C
C   Viscous incompressible fluids:
C   Compute coefficients associated with viscous terms (full integ.)_____
C
            ELK(II,JJ) = ELK(II,JJ) + VIISCSITY*(2.0*S11 + S22)
            ELK(II+1,JJ) = ELK(II+1,JJ) + VIISCSITY*S12
            ELK(II,JJ+1) = ELK(II,JJ+1) + VIISCSITY*S21
            ELK(II+1,JJ+1) = ELK(II+1,JJ+1) + VIISCSITY*(S11 + 2.0*S22)
            IF (ITEM.NE.0) THEN
              ELM(II,JJ) = ELM(II,JJ) + CT*S00
              ELM(II+1,JJ+1) = ELM(II+1,JJ+1) + CT*S00
            ENDIF
          ELSE
C
C   Plane elasticity problems:_____
C
            ELK(II,JJ) = ELK(II,JJ) + CMAT(1,1)*S11+CMAT(3,3)*S22
            ELK(II,JJ+1) = ELK(II,JJ+1) + CMAT(1,2)*S12+CMAT(3,3)*S21
            ELK(II+1,JJ) = ELK(II+1,JJ) + CMAT(1,2)*S21+CMAT(3,3)*S12
            ELK(II+1,JJ+1) = ELK(II+1,JJ+1)+CMAT(3,3)*S11+CMAT(2,2)*S22
            IF (ITEM.NE.0) THEN
              ELM(II,JJ) = ELM(II,JJ) + CT*S00
              ELM(II+1,JJ+1) = ELM(II+1,JJ+1) + CT*S00
            ENDIF
          ENDIF
        ENDIF
      ENDIF
    ENDIF
  ENDIF
ENDIF

```

```

        ENDIF
    ENDIF
ENDIF
160 JJ = NDF*I+1
IF(NEIGN.EQ.0) THEN
C      Source of the form fx = F0 + FX*X + FY*Y is assumed
C
C      L=(I-1)*NDF+1
C      ELF(L) = ELF(L)+CNST*SF(I)*SOURCE
C
ENDIF
180 II = NDF*I+1
200 CONTINUE
C
C      IF(ITYPE.EQ.1 .OR. ITYPE.EQ.3) THEN
C
C      Use reduced integration to evaluate coefficients associated with
C      penalty terms for flows and transverse shear terms for plates.
C
C      Call subroutine QUADRature for TRIangles to compute arrays of integration
C      points and weights for the given NIPR and IPDR
C
C      CALL QUADRTRI (NIPR,IPDR)
C
DO 280 NI=1,NIPR
AC1 = AL1(NI,IPDR)
AC2 = AL2(NI,IPDR)
AC3 = AL3(NI,IPDR)
CALL SHAPETRI(NPE,AC1,AC2,AC3,DET,ELXY)
CNST = 0.50D0*DET*ALWT(NI,IPDR)
C
II=1
DO 260 I=1,NPE
JJ = 1
DO 240 J=1,NPE
S11=GDSF(1,I)*GDSF(1,J)*CNST
S22=GDSF(2,I)*GDSF(2,J)*CNST
S12=GDSF(1,I)*GDSF(2,J)*CNST
S21=GDSF(2,I)*GDSF(1,J)*CNST
IF(ITYPE.EQ.1) THEN
C
C      Viscous incompressible fluids (penalty terms): _____
C
ELK(II,JJ) = ELK(II,JJ) + PENALTY*S11
ELK(II+1,JJ) = ELK(II+1,JJ) + PENALTY*S21
ELK(II,JJ+1) = ELK(II,JJ+1) + PENALTY*S12
ELK(II+1,JJ+1) = ELK(II+1,JJ+1) + PENALTY*S22
ELSE
C
C      Shear deformable plates (transverse shear terms): _____
C
S00=SF(I)*SF(J)*CNST
S10 = GDSF(1,I)*SF(J)*CNST
S01 = SF(I)*GDSF(1,J)*CNST
S20 = GDSF(2,I)*SF(J)*CNST
S02 = SF(I)*GDSF(2,J)*CNST
ELK(II,JJ) = ELK(II,JJ) + C55*S11+C44*S22
ELK(II,JJ+1) = ELK(II,JJ+1) + C55*S10
ELK(II+1,JJ) = ELK(II+1,JJ) + C55*S01
ELK(II,JJ+2) = ELK(II,JJ+2) + C44*S20
ELK(II+2,JJ) = ELK(II+2,JJ) + C44*S02
ELK(II+1,JJ+1) = ELK(II+1,JJ+1) + C55*S00
ELK(II+2,JJ+2) = ELK(II+2,JJ+2) + C44*S00
ENDIF
240 JJ=NDF*I+1
260 II=NDF*I+1
280 CONTINUE
ENDIF
RETURN
END

```

SUBROUTINE JACOBI (N,Q,JVEC,M,V,X,IH,MXNEQ)

C Called in EGNSOLVR to diagonalize [Q] by successive rotations

```

C
C      DESCRIPTION OF THE VARIABLES:
C
C      N      .... Order of the real, symmetric matrix [Q] (N > 2)
C      [Q]    .... The matrix to be diagonalized (destroyed)
C      JVEC   .... 0, when only eigenvalues alone have to be found
C      [V]    .... Matrix of eigenvectors
C      M      .... Number of rotations performed
C
C
C      IMPLICIT REAL*8 (A-H,O-Z)
C      DIMENSION Q(MXNEQ,MXNEQ) , V(MXNEQ,MXNEQ) , X(MXNEQ) , IH(MXNEQ)
C      EPSI=1.0D-08
C
C      IF(JVEC)10,50,10
10     DO 40 I=1,N
        DO 40 J=1,N
          IF(I-J)30,20,30
20     V(I,J)=1.0
        GO TO 40
30     V(I,J)=0.0
40     CONTINUE
50     M=0
      MI=N-1
      DO 70 I=1,MI
        X(I)=0.0
      MJ=I+1
      DO 70 J=MJ,N
        IF(X(I)-DABS(Q(I,J)))60,60,70
60     X(I)=DABS(Q(I,J))
        IH(I)=J
70     CONTINUE
75     DO 100 I=1,MI
      IF(I-1)90,90,80
80     IF(XMAX-X(I))90,100,100
90     XMAX=X(I)
      IP=I
      JP=IH(I)
100    CONTINUE
      IF(XMAX-EPSI)500,500,110
110    M=M+1
      IF(Q(IP,IP)-Q(JP,JP))120,130,130
120    TANG=-2.0*Q(IP,JP)/(DABS(Q(IP,IP)-Q(JP,JP))+DSQRT((Q(IP,IP)
1           -Q(JP,JP))**2+4.0*Q(IP,JP)**2))
        GO TO 140
130    TANG= 2.0*Q(IP,JP)/(DABS(Q(IP,IP)-Q(JP,JP))+DSQRT((Q(IP,IP)
1           -Q(JP,JP))**2+4.0*Q(IP,JP)**2))
140    COSN=1.0/DSQRT(1.0+TANG**2)
      SINE=TANG*COSN
      QII=Q(IP,IP)
      Q(IP,IP)=COSN**2*(QII+TANG*(2.*Q(IP,JP)+TANG*Q(JP,JP)))
      Q(JP,JP)=COSN**2*(Q(JP,JP)-TANG*(2.*Q(IP,JP)-TANG*QII))
      Q(IP,JP)=0.0
      IF (Q(IP,IP)-Q(JP,JP)) 150,190,190
150    TEMP=Q(IP,IP)
      Q(IP,IP)=Q(JP,JP)
      Q(JP,JP)=TEMP
      IF(SINE) 160,170,170
160    TEMP=COSN
      GOTO 180
170    TEMP=-COSN
180    COSN=DABS(SINE)
      SINE=TEMP
190    DO 260 I=1,MI
      IF (I-IP) 210,260,200
200    IF (I-JP) 210,260,210
210    IF (IH(I)-IP) 220,230,220
220    IF (IH(I)-JP) 260,230,260
230    K=IH(I)
      TEMP=Q(I,K)
      Q(I,K)=0.0
      MJ=I+1
      X(I)=0.0
      DO 250 J=MJ,N
        IF (X(I)-DABS(Q(I,J))) 240,240,250
240    X(I)=DABS(Q(I,J))

```

```

IH(I)=J
250 CONTINUE
Q(I,K)=TEMP
260 CONTINUE
X(IP)=0.0
X(JP)=0.0
DO 430 I=1,N
IF (I-IP) 270,430,320
270 TEMP=Q(I,IP)
Q(I,IP)=COSN*TEMP+SINE*Q(I,JP)
IF (X(I)-DABS(Q(I,IP))) 280,290,290
280 X(I)=DABS(Q(I,IP))
IH(I)=IP
290 Q(I,JP)=-SINE*TEMP+COSN*Q(I,JP)
IF (X(I)-DABS(Q(I,JP))) 300,430,430
300 X(I)=DABS(Q(I,JP))
IH(I)=JP
GO TO 430
320 IF (I-JP) 330,430,380
330 TEMP=Q(IP,I)
Q(IP,I)=COSN*TEMP+SINE*Q(IP,JP)
IF (X(IP)-DABS(Q(IP,I))) 340,350,350
340 X(IP)=DABS(Q(IP,I))
IH(IP)=I
350 Q(IP,JP)=-SINE*TEMP+COSN*Q(IP,JP)
IF (X(IP)-DABS(Q(IP,JP))) 300,430,430
380 TEMP=Q(IP,I)
Q(IP,I)=COSN*TEMP+SINE*Q(JP,I)
IF (X(IP)-DABS(Q(IP,I))) 390,400,400
390 X(IP)=DABS(Q(IP,I))
IH(IP)=I
400 Q(JP,I)=-SINE*TEMP+COSN*Q(JP,I)
IF (X(JP)-DABS(Q(JP,I))) 410,430,430
410 X(JP)=DABS(Q(JP,I))
IH(JP)=I
430 CONTINUE
IF (JVEC) 440,75,440
440 DO 450 I=1,N
TEMP=V(I,IP)
V(I,IP)=COSN*TEMP+SINE*V(I,JP)
450 V(I,JP)=-SINE*TEMP+COSN*V(I,JP)
GOTO 75
500 RETURN
END

```

SUBROUTINE MESH2DG(NELEM,NNODE,NOD,MAXELM,MAXNOD,GLXY)

C
C
C Called in MAIN to generate nodal point coordinates for specified
C type meshes (see Fig. 13.4.2 for examples)
C
C NOD1 = First node number in the line segment
C NODL = Last node number in the line segment
C NODINC= Node increment from one node to the next along the line
C X1,Y1 = Global coordinates of the first node on the line
C XL,YL = Global coordinates of the last node on the line
C RATIO = The ratio of the first element to the last element
C
C
IMPLICIT REAL*8 (A-H,O-Z)
DIMENSION GLXY(MAXNOD,2),NOD(MAXELM,9)
C
DO 10 I=1,NNODE
GLXY(I,1)=1.E20
10 GLXY(I,2)=1.E20
C
C Read number of the records (line segments) and data in each line
C
READ(5,*) NRECL
DO 30 IREC=1,NRECL
READ(5,*) NOD1,NODL,NODINC,X1,Y1,XL,YL,RATIO
IF(NODL.LT.NOD1) NODL = NOD1
IF(NODL.NE.NOD1) THEN
IF(NODINC.LE.0) NODINC = 1
IF(RATIO.LE.0.0) RATIO=1.0

```

NODIF = (NODL-NOD1)/NODINC
XL1=XL-X1
YL1=YL-Y1
GLXY(NOD1,1)=X1
GLXY(NOD1,2)=Y1
ALNGTH=DSQRT(XL1*XL1+YL1*YL1)
ALINC=(2.0*ALNGTH/NODIF)*RATIO/(RATIO+1.0)
ALRAT=ALINC/RATIO
IF(NODIF.NE.1) DEL=(ALINC-ALRAT)/(NODIF-1)
IF(NODIF.EQ.1) DEL=0.0
SUM=0.0
I=-1
DO 20 N=1,NODIF
I=I+1
SUM=SUM+ALINC-I*DEL
NI=NOD1+N*NODINC
GLXY(NI,1)=X1+XL1*SUM/ALNGTH
GLXY(NI,2)=Y1+YL1*SUM/ALNGTH
20 CONTINUE
ENDIF
30 CONTINUE
CALL CONCTVTY(NELEM,NOD,MAXELM,MAXNOD,GLXY)
RETURN
END

```

```

SUBROUTINE MESH2DR(IEL,IETYP,NX,NY,NPE,NNM,NEM,NOD,DX,DY,X0,Y0,
1 GLXY,MAXELM,MAXNOD,MAXNX,MAXNY)

```

C
C
Called in MAIN to compute arrays [NOD] & [GLXY] for rectangular
C domains. The domain is divided into NX subdivisions along the
C x-direction and NY subdivisions in the y-direction. The subdivi-
C sions define rectangular elements of the type required. For a
C triangular element mesh, the subdivision defines two linear ele-
C ments per a rectangular element with their common diagonal being
C inclined to the right (see Fig. 13.4.1 of the text).
C

```

IMPLICIT REAL*8 (A-H,O-Z)
DIMENSION NOD(MAXELM,9),GLXY(MAXNOD,2),DX(MAXNX),DY(MAXNY)
COMMON/IO/IN,ITT

```

```

C
NEX1 = NX+1
NEY1 = NY+1
NXX = IEL*NX
NYY = IEL*NY
NXX1 = NXX + 1
NYY1 = NYY + 1
NEM = NX*NY
IF(IETYP.EQ.0)NEM=2*NX*NY
NNM=NXX1*NYY1
IF(NPE.EQ.8)NNM = NXX1*NYY1 - NX*NY
IF(IETYP.EQ.0) THEN

```

```

C
C Generate the array [NOD] : _____
C

```

```

TRIANGULAR ELEMENTS
C
NX2=2*NX
NY2=2*NY
NOD(1,1) = 1
NOD(1,2) = IEL+1
NOD(1,3) = IEL*NXX1+IEL+1
IF(NPE.GT.3) THEN
    NOD(1,4) = 2
    NOD(1,5) = NXX1 + 3
    NOD(1,6) = NXX1 + 2
ENDIF
NOD(2,1) = 1
NOD(2,2) = NOD(1,3)
NOD(2,3) = IEL*NXX1+1
IF(NPE.GT.3) THEN
    NOD(2,4) = NOD(1,6)
    NOD(2,5) = NOD(1,3) - 1
    NOD(2,6) = NOD(2,4) - 1
ENDIF

```

```

K=3
DO 60 IY=1,NY
L=IY*NX2
M=(IY-1)*NX2
IF(NX.GT.1) THEN
DO 30 N=K,L,2
DO 20 I=1,NPE
NOD(N,I) = NOD(N-2,I)+IEL
NOD(N+1,I) = NOD(N-1,I)+IEL
20
30
CONTINUE
ENDIF
IF(IY.LT.NY) THEN
DO 40 I=1,NPE
NOD(L+1,I)=NOD(M+1,I)+IEL*NXX1
40
NOD(L+2,I)=NOD(M+2,I)+IEL*NXX1
ENDIF
60
K=L+3
ELSE
C
C      RECTANGULAR ELEMENTS
C
K0 = 0
IF(NPE .EQ. 9) K0=1
NOD(1,1) = 1
NOD(1,2) = IEL+1
NOD(1,3) = NXX1+(IEL-1)*NEX1+IEL+1
IF(NPE .EQ. 9) NOD(1,3)=4*NX+5
NOD(1,4) = NOD(1,3) - IEL
IF(NPE .GT. 4) THEN
NOD(1,5) = 2
NOD(1,6) = NXX1 + (NPE-6)
NOD(1,7) = NOD(1,3) - 1
NOD(1,8) = NXX1+1
IF(NPE .EQ. 9) THEN
NOD(1,9)=NXX1+2
ENDIF
ENDIF
IF(NY .GT. 1) THEN
M = 1
DO 110 N = 2,NY
L = (N-1)*NX + 1
DO 100 I = 1,NPE
100
110
NOD(L,I) = NOD(M,I)+NXX1+(IEL-1)*NEX1+K0*NX
M=L
ENDIF
C
IF(NX .GT. 1) THEN
DO 140 NI = 2,NX
DO 120 I = 1,NPE
K1 = IEL
IF(I .EQ. 6 .OR. I .EQ. 8) K1=1+K0
120
NOD(NI,I) = NOD(NI-1,I)+K1
M = NI
DO 140 NJ = 2,NY
L = (NJ-1)*NX+NI
DO 130 J = 1,NPE
130
140
NOD(L,J) = NOD(M,J)+NXX1+(IEL-1)*NEX1+K0*NX
M = L
ENDIF
ENDIF
C
C      Generate the global coordinates of the nodes, [GLXY] : _____
C
DX(NEX1)=0.0
DY(NEY1)=0.0
XC=X0
YC=Y0
IF(NPE .EQ. 8) THEN
DO 180 NI = 1, NEY1
I = (NXX1+NEX1)*(NI-1)+1
J = 2*NI-1
GLXY(I,1) = XC
GLXY(I,2) = YC
DO 150 NJ = 1,NX
DELX=0.5*DX(NJ)
I=I+1

```

```

GLXY(I,1) = GLXY(I-1,1)+DELX
GLXY(I,2) = YC
I=I+1
GLXY(I,1) = GLXY(I-1,1)+DELX
GLXY(I,2) = YC
150 CONTINUE
IF(NI.LE.NY) THEN
  I = I+1
  YC= YC+0.5*DY(NI)
  GLXY(I,1) = XC
  GLXY(I,2) = YC
  DO 160 II = 1, NX
  I = I+1
  GLXY(I,1) = GLXY(I-1,1)+DX(II)
  GLXY(I,2) = YC
160 ENDIF
180 YC = YC+0.5*DY(NI)

C
ELSE
YC=Y0
DO 200 NI = 1, NEY1
XC = X0
I = NXX1*IEL*(NI-1)
DO 190 NJ = 1, NEX1
I=I+1
GLXY(I,1) = XC
GLXY(I,2) = YC
IF(NJ.LT.NEX1) THEN
  IF(IEL.EQ.2) THEN
    I=I+1
    XC = XC + 0.5*DX(NJ)
    GLXY(I,1) = XC
    GLXY(I,2) = YC
  ENDIF
ENDIF
190 XC = XC + DX(NJ)/IEL
XC = X0
IF(IEL.EQ.2) THEN
  YC = YC + 0.5*DY(NI)
  DO 195 NJ = 1, NEX1
  I=I+1
  GLXY(I,1) = XC
  GLXY(I,2) = YC
  IF(NJ.LT.NEX1) THEN
    I=I+1
    XC = XC + 0.5*DX(NJ)
    GLXY(I,1) = XC
    GLXY(I,2) = YC
  ENDIF
  XC = XC + 0.5*DX(NJ)
195 ENDIF
200 YC = YC + DY(NI)/IEL
ENDIF
RETURN
END

```

SUBROUTINE MATRXMLT(MXNEQ,N,A,B,C)

C
C
C Called in EGNSOLVR to computer the product of matrices [A] & [B] :
C [C]=[A][B]
C

```

IMPLICIT REAL*8 (A-H,O-Z)
DIMENSION A(MXNEQ,MXNEQ),B(MXNEQ,MXNEQ),C(MXNEQ,MXNEQ)
DO 10 I=1,N
DO 10 J=1,N
C(I,J)=0.0
DO 10 K=1,N
10 C(I,J)=C(I,J)+A(I,K)*B(K,J)
RETURN
END

```

SUBROUTINE POSTPROC(ELXY,ITYPE,IELTYP,IGRAD,NDF,NPE,THKNS,ELU,

```

*          ISTR,NSTR)
C
C
C   Called in MAIN to compute the derivatives of the solution for
C   heat transfer and like problems, and stresses for fluid flow,
C   plane elasticity and plate bending problems.
C
C
IMPLICIT REAL*8 (A-H,O-Z)
DIMENSION ELXY(9,2),ELU(27),GAUSPT(4,4)
COMMON/PST/A10,A1X,A1Y,A20,A2X,A2Y,A00,C0,CX,CY,F0,FX,FY,
1           C44,C55,VISCSITY,PENALTY,CMAT(3,3)
COMMON/SHP/SF(9),GDSF(2,9),SFH(16),GDSFH(2,16),GDDSFH(3,16)
COMMON/QUAD/AL1(7,5),AL2(7,5),AL3(7,5),ALWT(7,5)
COMMON/IO/IN,ITT
C
DATA GAUSPT/4*0.0D0, -0.57735027D0, 0.57735027D0, 2*0.0D0,
2      -0.77459667D0, 0.0D0, 0.77459667D0, 0.0D0, -0.86113631D0,
3      -0.33998104D0, 0.33998104D0, 0.86113631D0/
C
PI=4.0D0*DATAN(1.0D0)
CONST=180.0D0/PI
IF (IELTYP.EQ.0) THEN
C
C   Computation of the gradient/stresses at the reduced-integration
C   points of TRIANGULAR ELEMENTS:
C
CALL QUADRTRI (NSTR,ISTR)
C
DO 40 NI=1,NSTR
AC1 = AL1(NI,ISTR)
AC2 = AL2(NI,ISTR)
AC3 = AL3(NI,ISTR)
CALL SHAPETRI(NPE,AC1,AC2,AC3,DET,ELXY)
XC = 0.0
YC = 0.0
DO 10 I=1,NPE
XC = XC+SF(I)*ELXY(I,1)
YC = YC+SF(I)*ELXY(I,2)
IF (ITYPE.LT.3) THEN
UX = 0.0
UY = 0.0
VX = 0.0
VY = 0.0
DO 20 I=1,NPE
J=NDF*I-1
IF (ITYPE.EQ.0) J=I
UX = UX + ELU(J)*GDSF(1,I)
UY = UY + ELU(J)*GDSF(2,I)
IF (ITYPE.GE.1) THEN
K=J+1
VX = VX + ELU(K)*GDSF(1,I)
VY = VY + ELU(K)*GDSF(2,I)
ENDIF
20 CONTINUE
C
IF (ITYPE.EQ.0) THEN
C
C   Single-degree-of-freedom problems:-----
C
SX = -(A10+A1X*XC+A1Y*YC)*UX
SY = -(A20+A2X*XC+A2Y*YC)*UY
VALUE= DSQRT(SX**2+SY**2)
IF (IGRAD.EQ.1) THEN
QX=SX
QY=SY
ELSE
QX=-SY
QY= SX
ENDIF
IF (QX.EQ.0.0) THEN
IF (QY.LT.0.0) THEN
ANGLE = -90.0
ELSE
ANGLE = 90.0
ENDIF

```

```

        ELSE
            ANGLE=DATAN2 (QY,QX) *CONST
        ENDIF
        WRITE(ITT,200) XC,YC,QX,QY,VALUE,ANGLE
    ELSE
C
        IF(ITYPE.EQ.1) THEN
C
C      Viscous incompressible flows (penalty model):-----
C
        PRESSR = -PENALTY*(UX+VY)
        STRESX = 2.0*VISCSITY*UX-PRESSR
        STRESY = 2.0*VISCSITY*VY-PRESSR
        STRSXY = VISCSITY*(UY+VX)
        WRITE(ITT,300) XC,YC,STRESX,STRESY,STRSXY,PRESSR
    ELSE
C
        Plane elasticity problems:-----
C
        STRESX = (CMAT(1,1)*UX+CMAT(1,2)*VY)/THKNS
        STRESY = (CMAT(1,2)*UX+CMAT(2,2)*VY)/THKNS
        STRSXY = CMAT(3,3)*(UY+VX)/THKNS
        WRITE(ITT,300) XC,YC,STRESX,STRESY,STRSXY
    ENDIF
    ENDIF
ENDIF
40    CONTINUE
ELSE
C
C      Calculation of the gradient/stresses at the reduced integration
C      gauss points of RECTANGULAR ELEMENTS: -----
C
DO 100 NI=1,ISTR
DO 100 NJ=1,ISTR
XI = GAUSPT(NI,ISTR)
ETA = GAUSPT(NJ,ISTR)
CALL SHAPERCT (NPE,XI,ETA,DET,ELXY,NDF,ITYPE)
XC = 0.0
YC = 0.0
DO 50 I=1,NPE
XC = XC+SF(I)*ELXY(I,1)
YC = YC+SF(I)*ELXY(I,2)
IF(ITYPE.LT.3) THEN
    UX = 0.0
    UY = 0.0
    VX = 0.0
    VY = 0.0
DO 60 I=1,NPE
J=NDF*I-1
IF(ITYPE.EQ.0) J=I
UX = UX + ELU(J)*GDSF(1,I)
UY = UY + ELU(J)*GDSF(2,I)
IF(ITYPE.GE.1) THEN
    K=J+1
    VX = VX + ELU(K)*GDSF(1,I)
    VY = VY + ELU(K)*GDSF(2,I)
ENDIF
50    CONTINUE
IF(ITYPE.EQ.0) THEN
C
C      Single-degree-of-freedom problems: -----
C
SX = -(A10+A1X*XC+A1Y*YC)*UX
SY = -(A20+A2X*XC+A2Y*YC)*UY
VALUE= DSQRT(SX**2+SY**2)
IF(IGRAD.EQ.1) THEN
    QX=SX
    QY=SY
ELSE
    QX=-SY
    QY= SX
ENDIF
IF(QX.EQ.0.0) THEN
    IF(QY.LT.0.0) THEN
        ANGLE =-90.0
    ELSE

```

```

        ANGLE = 90.0
    ENDIF
    ELSE
        ANGLE=DATAN2 (QY , QX) *CONST
    ENDIF
    WRITE (ITT,200) XC,YC,QX,QY,VALUE,ANGLE
ELSE
C
    IF (ITYPE.EQ.1) THEN
C
C      Viscous incompressible flows (penalty model) :-----
C
        PRESSR = -PENALTY* (UX+VY)
        STRESX = 2.0*VISCSITY*UX-PRESSR
        STRESY = 2.0*VISCSITY*VY-PRESSR
        STRSXY = VISCSITY* (UY+VX)
        WRITE (ITT,300) XC,YC,STRESX,STRESY,STRSXY,PRESSR
    ELSE
C
C      Plane elasticity problems:-----
C
        STRESX = (CMAT(1,1)*UX+CMAT(1,2)*VY)/THKNS
        STRESY = (CMAT(1,2)*UX+CMAT(2,2)*VY)/THKNS
        STRSXY = CMAT(3,3)* (UY+VX) /THKNS
        WRITE (ITT,300) XC,YC,STRESX,STRESY,STRSXY
    ENDIF
    ENDIF
    ELSE
C
C      Plate bending problems:-----
C      Stresses SGMAX, SGMAX and SGMXY are computed at the top/bottom of
C      the plate (and SGMXZ and SGMYZ are constant through thickness)
C
        PLTD=(THKNS*THKNS) /6.0D0
        SIX = 0.0
        SIY = 0.0
        DWX = 0.0
        DWY = 0.0
        DSXY = 0.0
        DSYX = 0.0
        DSXX = 0.0
        DSYY = 0.0
        IF (ITYPE.EQ.3) THEN
C
C      First-order shear deformation theory of plates:-----
C
            DO 80 I=1,NPE
            J=NDF*(I-1)+1
            K=J+1
            L=K+1
            DWX = DWX+GDSF(1,I)*ELU(J)
            DWY = DWY+GDSF(2,I)*ELU(J)
            SIX = SIX+SF(I)*ELU(K)
            SIY = SIY+SF(I)*ELU(L)
            DSXX = DSXX+GDSF(1,I)*ELU(K)
            DSXY = DSXY+GDSF(2,I)*ELU(K)
            DSYX = DSYX+GDSF(1,I)*ELU(L)
            DSYY = DSYY+GDSF(2,I)*ELU(L)
80          SGMAX = (CMAT(1,1)*DSXX+CMAT(1,2)*DSYY) /PLTD
            SGMAX = (CMAT(1,2)*DSXX+CMAT(2,2)*DSYY) /PLTD
            SGMXY = CMAT(3,3)*(DSXY+DSYX) /PLTD
            SGMXZ = 1.2*C55*(DWX+SIX) /THKNS
            SGMYZ = 1.2*C44*(DWY+SIY) /THKNS
            WRITE (ITT,300) XC,YC,SGMAX,SGMAY,SGMXY
            WRITE (ITT,400) SGMXZ,SGMYZ
        ELSE
C
C      Classical theory of plates:-----
C
            NN=NPE*NDF
            DO 90 I=1,NN
            DSXX = DSXX+GDDSFH(1,I)*ELU(I)
            DSYY = DSYY+GDDSFH(2,I)*ELU(I)
            DSXY = DSXY+GDDSFH(3,I)*ELU(I)
90          SGMAX =-(CMAT(1,1)*DSXX+CMAT(1,2)*DSYY) /PLTD

```

```

        SGMAY =- (CMAT(1,2)*DSXX+CMAT(2,2)*DSYY)/PLTD
        SGMXY =-4.0*CMAT(3,3)*DSXY/PLTD
        WRITE(ITT,300) XC,YC,SGMAX,SGMAY,SGMXY
    ENDIF
ENDIF
100    CONTINUE
ENDIF
200 FORMAT(5E13.4,3X,F7.2)
300 FORMAT(6E13.4)
400 FORMAT(26X,2E13.4)
RETURN
END

```

SUBROUTINE QUADRTRI(NIP,IPD)

```

C
C
C Called in ELKMFTRI to compute the quadrature points and weights
C for triangular elements
C
C     IPD = Integrand Polynomial Degree
C     NIP = Number of Integration Points
C
C
C IMPLICIT REAL*8 (A-H,O-Z)
COMMON/QUAD/AL1(7,5),AL2(7,5),AL3(7,5),ALWT(7,5)
C
C Initialize arrays
C
DO 20 I = 1, NIP
DO 10 J = 1, IPD
AL1(I,J) = 0.0000000000000000
AL2(I,J) = 0.0000000000000000
AL3(I,J) = 0.0000000000000000
ALWT(I,J) = 0.0000000000000000
10 CONTINUE
20 CONTINUE
C
C One-point quadrature (for polynomials of order 1): _____
C
AL1(1,1) = 0.3333333333333333
AL2(1,1) = 0.3333333333333333
AL3(1,1) = 0.3333333333333333
ALWT(1,1) = 1.0000000000000000
C
C Three-point quadrature (for polynomials of order 2): _____
C
AL1(1,2) = 0.0000000000000000
AL2(1,2) = 0.5000000000000000
AL3(1,2) = 0.5000000000000000
AL1(2,2) = 0.5000000000000000
AL2(2,2) = 0.0000000000000000
AL3(2,2) = 0.5000000000000000
AL1(3,2) = 0.5000000000000000
AL2(3,2) = 0.5000000000000000
AL3(3,2) = 0.0000000000000000
ALWT(1,2) = 0.3333333333333333
ALWT(2,2) = 0.3333333333333333
ALWT(3,2) = 0.3333333333333333
C
C Four-point quadrature (for polynomials of order 3): _____
C
AL1(1,3) = 0.3333333333333333
AL2(1,3) = 0.3333333333333333
AL3(1,3) = 0.3333333333333333
AL1(2,3) = 0.6000000000000000
AL2(2,3) = 0.2000000000000000
AL3(2,3) = 0.2000000000000000
AL1(3,3) = 0.2000000000000000
AL2(3,3) = 0.6000000000000000
AL3(3,3) = 0.2000000000000000
AL1(4,3) = 0.2000000000000000
AL2(4,3) = 0.2000000000000000
AL3(4,3) = 0.6000000000000000
ALWT(1,3) = -0.5625000000000000
ALWT(2,3) = 0.5208333333333333

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ALWT(3,3) = 0.5208333333333333
ALWT(4,3) = 0.5208333333333333
C
C Six-point quadrature (for polynomials of order 4): _____
C
AL1(1,4) = 0.816847572980459
AL2(1,4) = 0.091576213509771
AL3(1,4) = 0.091576213509771
AL1(2,4) = 0.091576213509771
AL2(2,4) = 0.816847572980459
AL3(2,4) = 0.091576213509771
AL1(3,4) = 0.091576213509771
AL2(3,4) = 0.091576213509771
AL3(3,4) = 0.816847572980459
AL1(4,4) = 0.108103018168070
AL2(4,4) = 0.445948490915965
AL3(4,4) = 0.445948490915965
AL1(5,4) = 0.445948490915965
AL2(5,4) = 0.108103018168070
AL3(5,4) = 0.445948490915965
AL1(6,4) = 0.445948490915965
AL2(6,4) = 0.445948490915965
AL3(6,4) = 0.108103018168070
ALWT(1,4) = 0.109951743655322
ALWT(2,4) = 0.109951743655322
ALWT(3,4) = 0.109951743655322
ALWT(4,4) = 0.223381589678011
ALWT(5,4) = 0.223381589678011
ALWT(6,4) = 0.223381589678011
C
C Seven-point quadrature (for polynomials of order 5): _____
C
AL1(1,5) = 0.3333333333333333
AL2(1,5) = 0.3333333333333333
AL3(1,5) = 0.3333333333333333
AL1(2,5) = 0.797426985353087
AL2(2,5) = 0.101286507323456
AL3(2,5) = 0.101286507323456
AL1(3,5) = 0.101286507323456
AL2(3,5) = 0.797426985353087
AL3(3,5) = 0.101286507323456
AL1(4,5) = 0.101286507323456
AL2(4,5) = 0.101286507323456
AL3(4,5) = 0.797426985353087
AL1(5,5) = 0.059715871789770
AL2(5,5) = 0.470142064105115
AL3(5,5) = 0.470142064105115
AL1(6,5) = 0.470142064105115
AL2(6,5) = 0.059715871789770
AL3(6,5) = 0.470142064105115
AL1(7,5) = 0.470142064105115
AL2(7,5) = 0.470142064105115
AL3(7,5) = 0.059715871789770
ALWT(1,5) = 0.2250000000000000
ALWT(2,5) = 0.125939180544827
ALWT(3,5) = 0.125939180544827
ALWT(4,5) = 0.125939180544827
ALWT(5,5) = 0.132394152788506
ALWT(6,5) = 0.132394152788506
ALWT(7,5) = 0.132394152788506
C
RETURN
END

SUBROUTINE SHAPERCT(NPE,XI,ETA,DET,ELXY,NDF,ITYPE)
C
C
C Called in SHAPERCT to evaluate the interpolation functions SF(I)
C and the derivatives with respect to global coordinates GDSF(I,J)
C for Lagrange linear & quadratic rectangular elements, using the
C isoparametric formulation. The subroutine also evaluates Hermite
C interpolation functions and their global derivatives using the
C subparametric formulation.
C
SF(I).....Interpolation function for node I of the element

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C DSF(J,I) ....Derivative of SF(I) with respect to XI if J=1 and
C and ETA if J=2
C GDSF(J,I) ....Derivative of SF(I) with respect to X if J=1 and
C and Y if J=2
C XNODE(I,J) ...J-TH (J=1,2) Coordinate of node I of the element
C NP(I) .....Array of element nodes (used to define SF and DSF)
C GJ(I,J) .....Determinant of the Jacobian matrix
C GJINV(I,J) ...Inverse of the jacobian matrix
C
C
IMPLICIT REAL*8 (A-H,O-Z)
DIMENSION ELXY(9,2),XNODE(9,2),NP(9),DSF(2,9),GJ(2,2),GJINV(2,2),
DIMENSION GGF(3,3),GGINV(3,3),DDSF(3,16),DDSFH(3,16),DJCB(3,2),
* DSFH(3,16),DDSFH(3,16)
COMMON/SHP/SF(9),GDSF(2,9),SFH(16),GDSFH(2,16),GDDSFH(3,16)
COMMON/IO/IN,ITT
DATA XNODE/-1.0D0, 2*1.0D0, -1.0D0, 0.0D0, 1.0D0, 0.0D0, -1.0D0,
* 0.0D0, 2*-1.0D0, 2*1.0D0, -1.0D0, 0.0D0, 1.0D0, 2*0.0D0/
DATA NP/1,2,3,4,5,7,6,8,9/
C
FNC(A,B) = A*B
IF(NPE.EQ.4) THEN
C
C LINEAR Lagrange interpolation functions for FOUR-NODE element
C
DO 10 I = 1, NPE
XP = XNODE(I,1)
YP = XNODE(I,2)
XI0 = 1.0+XI*XP
ETA0=1.0+ETA*YP
SF(I) = 0.25*FNC(XI0,ETA0)
DSF(1,I)= 0.25*FNC(XP,ETA0)
DSF(2,I)= 0.25*FNC(YP,XI0)
10 ELSE
IF(NPE.EQ.8) THEN
C
C QUADRATIC Lagrange interpolation functions for EIGHT-NODE element
C
DO 20 I = 1, NPE
NI = NP(I)
XP = XNODE(NI,1)
YP = XNODE(NI,2)
XI0 = 1.0+XI*XP
ETA0 = 1.0+ETA*YP
XI1 = 1.0-XI*XI
ETA1 = 1.0-ETA*ETA
IF(I.LE.4) THEN
SF(NI) = 0.25*FNC(XI0,ETA0)*(XI*XP+ETA*YP-1.0)
DSF(1,NI) = 0.25*FNC(ETA0,XP)*(2.0*XI*XP+ETA*YP)
DSF(2,NI) = 0.25*FNC(XI0,YP)*(2.0*ETA*YP+XI*XP)
ELSE
IF(I.LE.6) THEN
SF(NI) = 0.5*FNC(XI1,ETA0)
DSF(1,NI) = -FNC(XI,ETA0)
DSF(2,NI) = 0.5*FNC(YP,XI1)
ELSE
SF(NI) = 0.5*FNC(ETA1,XI0)
DSF(1,NI) = 0.5*FNC(XP,ETA1)
DSF(2,NI) = -FNC(ETA,XI0)
ENDIF
ENDIF
20 CONTINUE
ELSE
C
C QUADRATIC Lagrange interpolation functions for NINE-NODE element
C
DO 30 I=1,NPE
NI = NP(I)
XP = XNODE(NI,1)
YP = XNODE(NI,2)
XI0 = 1.0+XI*XP
ETA0 = 1.0+ETA*YP
XI1 = 1.0-XI*XI
ETA1 = 1.0-ETA*ETA
XI2 = XP*XI
ETA2 = YP*ETA

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IF(I .LE. 4) THEN
  SF(NI) = 0.25*FNC(XI0,ETA0)*XI2*ETA2
  DSF(1,NI)= 0.25*XP*FNC(ETA2,ETA0)*(1.0+2.0*XI2)
  DSF(2,NI)= 0.25*YP*FNC(XI2,XI0)*(1.0+2.0*ETA2)
ELSE
  IF(I .LE. 6) THEN
    SF(NI) = 0.5*FNC(XI1,ETA0)*ETA2
    DSF(1,NI) = -XI*FNC(ETA2,ETA0)
    DSF(2,NI) = 0.5*FNC(XI1,YP)*(1.0+2.0*ETA2)
  ELSE
    IF(I .LE. 8) THEN
      SF(NI) = 0.5*FNC(ETA1,XI0)*XI2
      DSF(2,NI) = -ETA*FNC(XI2,XI0)
      DSF(1,NI) = 0.5*FNC(ETA1,XP)*(1.0+2.0*XI2)
    ELSE
      SF(NI) = FNC(XI1,ETA1)
      DSF(1,NI) = -2.0*XI*ETA1
      DSF(2,NI) = -2.0*ETA*XI1
    ENDIF
  ENDIF
ENDIF
CONTINUE
ENDIF
ENDIF
C
C Compute the Jacobian matrix [GJ] and its inverse [GJINV]
C
DO 40 I = 1,2
DO 40 J = 1,2
GJ(I,J) = 0.0
DO 40 K = 1,NPE
  40 GJ(I,J) = GJ(I,J) + DSF(I,K)*ELXY(K,J)
C
DET = GJ(1,1)*GJ(2,2)-GJ(1,2)*GJ(2,1)
GJINV(1,1) = GJ(2,2)/DET
GJINV(2,2) = GJ(1,1)/DET
GJINV(1,2) = -GJ(1,2)/DET
GJINV(2,1) = -GJ(2,1)/DET
C
IF(ITYPE.LE.3) THEN
C
Compute the derivatives of the interpolation functions with
C respect to the global coordinates (x,y): [GDSF]
C
DO 50 I = 1,2
DO 50 J = 1,NPE
GDSF(I,J) = 0.0
DO 50 K = 1, 2
  50 GDSF(I,J) = GDSF(I,J) + GJINV(I,K)*DSF(K,J)
ELSE
C
Conforming Hermite interpolation functions (four-node element)
C
IF(NDF.EQ.4) THEN
  II = 1
  DO 60 I = 1, NPE
    XP = XNODE(I,1)
    YP = XNODE(I,2)
    XI1 = XI*XP-1.0
    XI2 = XI1-1.0
    ETA1 = ETA*YP-1.0
    ETA2 = ETA1-1.0
    XI0 = (XI+XP)*(XI+XP)
    ETA0 = (ETA+YP)*(ETA+YP)
    XIPO = XI+XP
    XIP1 = 3.0*XI*XP+XP*XP
    XIP2 = 3.0*XI*XP+2.0*XP*XP
    YIPO = ETA+YP
    YIP1 = 3.0*ETA*YP+YP*YP
    YIP2 = 3.0*ETA*YP+2.0*YP*YP
C
    SFH(II) = 0.0625*FNC(ETA0,ETA2)*FNC(XI0,XI2)
    DSFH(1,II) = 0.0625*FNC(ETA0,ETA2)*XIPO*(XIP1-4.0)
    DSFH(2,II) = 0.0625*FNC(XI0,XI2)*YIPO*(YIP1-4.0)
    DDSFH(1,II) = 0.125*FNC(ETA0,ETA2)*(XIP2-2.0)
    DDSFH(2,II) = 0.125*FNC(XI0,XI2)*(YIP2-2.0)

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      DDSFH(3, II) = 0.0625 * (XIP1-4.0) * (YIP1-4.0) * XIP0 * YIP0
C
      SFH(II+1) = -0.0625 * XP * FNC(XIO, XI1) * FNC(ETA0, ETA2)
      DSFH(1, II+1) = -0.0625 * FNC(ETA0, ETA2) * XP * XIP0 * (XIP1-2.0)
      DSFH(2, II+1) = -0.0625 * FNC(XIO, XI1) * XP * YIP0 * (YIP1-4.)
      DDSFH(1, II+1) = -0.125 * FNC(ETA0, ETA2) * XP * (XIP2-1.0)
      DDSFH(2, II+1) = -0.125 * FNC(XIO, XI1) * (YIP2-2.0) * XP
      DDSFH(3, II+1) = -0.0625 * XP * XIP0 * (XIP1-2.) * (YIP1-4.) * YIP0
C
      SFH(II+2) = -0.0625 * YP * FNC(XIO, XI2) * FNC(ETA0, ETA1)
      DSFH(1, II+2) = -0.0625 * FNC(ETA0, ETA1) * YP * XIP0 * (XIP1-4.)
      DSFH(2, II+2) = -0.0625 * FNC(XIO, XI2) * YP * YIP0 * (YIP1-2.)
      DDSFH(1, II+2) = -0.125 * FNC(ETA0, ETA1) * YP * (XIP2-2.)
      DDSFH(2, II+2) = -0.125 * FNC(XIO, XI2) * YP * (YIP2-1.0)
      DDSFH(3, II+2) = -0.0625 * YP * YIP0 * (YIP1-2.) * (XIP1-4.0) * XIP0
C
      SFH(II+3) = 0.0625 * XP * YP * FNC(XIO, XI1) * FNC(ETA0, ETA1)
      DSFH(1, II+3) = 0.0625 * FNC(ETA0, ETA1) * XP * YP * (XIP1-2.) * XIP0
      DSFH(2, II+3) = 0.0625 * FNC(XIO, XI1) * XP * YP * (YIP1-2.) * YIP0
      DDSFH(1, II+3) = 0.125 * FNC(ETA0, ETA1) * XP * YP * (XIP2-1.)
      DDSFH(2, II+3) = 0.125 * FNC(XIO, XI1) * XP * YP * (YIP2-1.0)
      DDSFH(3, II+3) = 0.0625 * XP * YP * YIP0 * XIP0 * (YIP1-2.) * (XIP1-2.)
      II = I*NDF + 1
60      CONTINUE
      ELSE
C
      Non-conforming Hermite interpolation functions (Four-node element)
C
      II = 1
      DO 80 I = 1, NPE
          XP = XNODE(I, 1)
          YP = XNODE(I, 2)
          XIO = XI * XP
          ETA0 = ETA * YP
          XIP1 = XI0+1
          ETAP1 = ETA0+1
          XIM1 = XI0-1
          ETAM1 = ETA0-1
          XID = 3.0+2.0*XIO+ETA0-3.0*XI*XI-ETA*ETA-2.0*XI/XP
          ETAD = 3.0+XI0+2.0*ETA0-XI*XI-3.0*ETA*ETA-2.0*ETA/YP
          ETAXI = 4.0+2.0*(XI0+ETA0)-3.0*(XI*XI+ETA*ETA)
          * -2.0*(ETA/YP+XI/XP)
C
          SFH(II) = 0.125 * XIP1 * ETAP1 * (2.0+XI0+ETA0-XI*XI-ETA*ETA)
          DSFH(1, II) = 0.125 * XP * ETAP1 * XID
          DSFH(2, II) = 0.125 * YP * XIP1 * ETAD
          DDSFH(1, II) = 0.250 * XP * ETAP1 * (XP-3.0*XI-1.0/XP)
          DDSFH(2, II) = 0.250 * YP * XIP1 * (YP-3.0*ETA-1.0/YP)
          DDSFH(3, II) = 0.125 * XP * YP * ETAXI
C
          SFH(II+1) = 0.125 * XP * XIP1 * XIP1 * XIM1 * ETAP1
          DSFH(1, II+1) = 0.125 * XP * XP * ETAP1 * (3.0*XI0-1.0) * XIP1
          DSFH(2, II+1) = 0.125 * XP * YP * XIP1 * XIP1 * XIM1
          DDSFH(1, II+1) = 0.250 * XP * XP * XP * ETAP1 * (3.0*XI0+1.0)
          DDSFH(2, II+1) = 0.0
          DDSFH(3, II+1) = 0.125 * XP * XP * YP * (3.0*XI0-1.0) * XIP1
C
          SFH(II+2) = 0.125 * YP * XIP1 * ETAP1 * ETAP1 * ETAM1
          DSFH(1, II+2) = 0.125 * XP * YP * ETAP1 * ETAP1 * ETAM1
          DSFH(2, II+2) = 0.125 * YP * YP * XIP1 * (3.0*ETA0-1.0) * ETAP1
          DDSFH(1, II+2) = 0.0
          DDSFH(2, II+2) = 0.250 * YP * YP * YP * XIP1 * (3.0*ETA0+1.0)
          DDSFH(3, II+2) = 0.125 * XP * YP * YP * (3.0*ETA0-1.0) * ETAP1
          II = I*NDF + 1
80      CONTINUE
      ENDIF
C
      Compute the global first and second derivatives of the Hermite
      interpolation functions. The geometry is approximated using the
      linear Lagrange interpolation functions (Subparametric formulation)
C
      DDSF(1, 1) = 0.0D0
      DDSF(2, 1) = 0.0D0
      DDSF(3, 1) = 0.250D0
      DDSF(1, 2) = 0.0D0
      DDSF(2, 2) = 0.0D0

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DDSF(3,2) = - 0.250D0
DDSF(1,3) = 0.0D0
DDSF(2,3) = 0.0D0
DDSF(3,3) = 0.250D0
DDSF(1,4) = 0.0D0
DDSF(2,4) = 0.0D0
DDSF(3,4) = - 0.250D0

C
C Compute global first derivatives of Hermite functions
C
      NN=NDF*NPE
      DO 110 I = 1, 2
      DO 100 J = 1, NN
         SUM = 0.0D0
         DO 90 K = 1, 2
            SUM = SUM + GJINV(I,K)*DSFH(K,J)
90      CONTINUE
         GDSFH(I,J) = SUM
100     CONTINUE
110     CONTINUE

C
C Compute global second derivatives of Hermite functions
C
      DO 140 I = 1, 3
      DO 130 J = 1, 2
         SUM = 0.0D0
         DO 120 K = 1, NPE
            SUM = SUM + DDSF(I,K)*ELXY(K,J)
120      CONTINUE
         DJCB(I,J) = SUM
130      CONTINUE
140      CONTINUE

C
      DO 170 K = 1, 3
      DO 160 J = 1, NN
         SUM = 0.0D0
         DO 150 L = 1, 2
            SUM = SUM + DJCB(K,L)*GDSFH(L,J)
150      CONTINUE
         DDSJ(K,J) = SUM
160      CONTINUE
170      CONTINUE

C
C Compute the jacobian of the transformation
C
      GGG(1,1)=GJ(1,1)*GJ(1,1)
      GGG(1,2)=GJ(1,2)*GJ(1,2)
      GGG(1,3)=2.0*GJ(1,1)*GJ(1,2)
      GGG(2,1)=GJ(2,1)*GJ(2,1)
      GGG(2,2)=GJ(2,2)*GJ(2,2)
      GGG(2,3)=2.0*GJ(2,1)*GJ(2,2)
      GGG(3,1)=GJ(2,1)*GJ(1,1)
      GGG(3,2)=GJ(2,2)*GJ(1,2)
      GGG(3,3)=GJ(2,1)*GJ(1,2)+GJ(1,1)*GJ(2,2)
      CALL INVERSE(GGG,GGINV)

C
      DO 200 I = 1, 3
      DO 190 J = 1, NN
         SUM = 0.0D0
         DO 180 K = 1, 3
            SUM = SUM + GGINV(I,K)*(DDSFH(K,J)-DDSJ(K,J))
180      CONTINUE
         GDDSFH(I,J) = SUM
190      CONTINUE
200      CONTINUE

      ENDIF
      RETURN
      END

SUBROUTINE SHAPETRI(NPE,AL1,AL2,AL3,DET,ELXY)

```

C
C Called in ELKMFTRI to evaluate the interpolation functions and
C their global derivatives at the quadrature points for the linear
C and quadratic (i.e., 3-node and 6-node) triangular elements.

```

C
C
IMPLICIT REAL*8 (A-H,O-Z)
COMMON/SHP/SF(9),GDSF(2,9),SFH(16),GDSFH(2,16),GDDSFH(3,16)
DIMENSION DSF(3,9),ELXY(9,2),GJ(2,2),GJINV(2,2)
C
C Initialize the arrays
C
DO 10 I = 1, NPE
DSF(1,I) = 0.0D0
DSF(2,I) = 0.0D0
DSF(3,I) = 0.0D0
10 CONTINUE
C
IF(NPE.EQ.3) THEN
C
Linear Lagrane interpolation for three-node element
C
SF(1) = AL1
SF(2) = AL2
SF(3) = AL3
DSF(1,1) = 1.0D0
DSF(2,2) = 1.0D0
DSF(3,3) = 1.0D0
ELSE
C
Quadratic Lagrange interpolation functions for six-nde element
C
SF(1) = AL1 * (2.0D0 * AL1 - 1)
SF(2) = AL2 * (2.0D0 * AL2 - 1)
SF(3) = AL3 * (2.0D0 * AL3 - 1)
SF(4) = 4.0D0 * AL1 * AL2
SF(5) = 4.0D0 * AL2 * AL3
SF(6) = 4.0D0 * AL3 * AL1
DSF(1,1) = 4.0D0 * AL1 - 1
DSF(2,2) = 4.0D0 * AL2 - 1
DSF(3,3) = 4.0D0 * AL3 - 1
DSF(1,4) = 4.0D0 * AL2
DSF(2,4) = 4.0D0 * AL1
DSF(2,5) = 4.0D0 * AL3
DSF(3,5) = 4.0D0 * AL2
DSF(1,6) = 4.0D0 * AL3
DSF(3,6) = 4.0D0 * AL1
ENDIF
C
Compute the global derivatives of SF(I). Note that the special
C form of the jacobian for area coordinates, AL3 = 1-AL1-AL2 is
C substituted
C
DO 60 I = 1,2
DO 50 J = 1,2
SUM = 0.0D0
DO 40 K = 1, NPE
SUM = SUM + (DSF(I,K) - DSF(3,K))*ELXY(K,J)
40 CONTINUE
GJ(I,J) = SUM
50 CONTINUE
60 CONTINUE
C
DET = GJ(1,1)*GJ(2,2) - GJ(1,2)*GJ(2,1)
GJINV(1,1) = GJ(2,2)/DET
GJINV(2,2) = GJ(1,1)/DET
GJINV(1,2) = -GJ(1,2)/DET
GJINV(2,1) = -GJ(2,1)/DET
DO 100 I = 1, 2
DO 90 J = 1, NPE
SUM = 0.0D0
DO 80 K = 1, 2
SUM = SUM + GJINV(I,K) * (DSF(K,J) - DSF(3,J))
80 CONTINUE
GDSF(I,J) = SUM
90 CONTINUE
100 CONTINUE
RETURN
END

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SUBROUTINE EQNSOLVR (NRM, NCM, NEQNS, NBW, BAND, RHS, IRES)
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```
C  
C  
C Called in MAIN to solve a banded, symmetric, system of algebraic  
C equations using the Gauss elimination method: [BAND] {U} = {RHS}.  
C The coefficient matrix is input as BAND(NEQNS,NBW) and the column  
C vector is input as RHS(NEQNS), where NEQNS is the actual number  
C of equations and NBW is the half band width. The true dimensions  
C of the matrix [BAND] in the calling program, are NRM by NCM. When  
C IRES is greater than zero, the right hand elimination is skipped.  
C
```

```
IMPLICIT REAL*8 (A-H,O-Z)  
DIMENSION BAND (NRM,NCM) , RHS (NRM)  
  
MEQNS=NEQNS-1  
IF (IRES.LE.0) THEN  
  DO 30 NPIV=1,MEQNS  
    NPIVOT=NPIV+1  
    LSTSUB=NPIV+NBW-1  
    IF (LSTSUB.GT.NEQNS) THEN  
      LSTSUB=NEQNS  
    ENDIF  
  
  DO 20 NROW=NPIVOT,LSTSUB  
    NCOL=NROW-NPIV+1  
    FACTOR=BAND (NPIV, NCOL) /BAND (NPIV, 1)  
    DO 10 NCOL=NROW,LSTSUB  
      ICOL=NCOL-NROW+1  
      JCOL=NCOL-NPIV+1  
      10 BAND (NROW, ICOL) =BAND (NROW, ICOL) -FACTOR*BAND (NPIV, JCOL)  
    20 RHS (NROW) =RHS (NROW) -FACTOR*RHS (NPIV)  
    30 CONTINUE  
  ELSE  
    DO 40 NPIV=1,MEQNS  
      NPIVOT=NPIV+1  
      LSTSUB=NPIV+NBW-1  
      IF (LSTSUB.GT.NEQNS) THEN  
        LSTSUB=NEQNS  
      ENDIF  
      DO 50 NROW=NPIVOT,LSTSUB  
        NCOL=NROW-NPIV+1  
        FACTOR=BAND (NPIV, NCOL) /BAND (NPIV, 1)  
      50 RHS (NROW) =RHS (NROW) -FACTOR*RHS (NPIV)  
    60 CONTINUE  
  ENDIF  
  
C Back substitution  
C  
  DO 90 IJK=2,NEQNS  
    NPIV=NEQNS-IJK+2  
    RHS (NPIV) =RHS (NPIV) /BAND (NPIV, 1)  
    LSTSUB=NPIV-NBW+1  
    IF (LSTSUB.LT.1) THEN  
      LSTSUB=1  
    ENDIF  
    NPIVOT=NPIV-1  
    DO 80 JKI=LSTSUB,NPIVOT  
      NROW=NPIVOT-JKI+LSTSUB  
      NCOL=NPIV-NROW+1  
      FACTOR=BAND (NROW, NCOL)  
    80 RHS (NROW) =RHS (NROW) -FACTOR*RHS (NPIV)  
  90 CONTINUE  
  RHS (1)=RHS (1) /BAND (1, 1)  
  RETURN  
END
```

```
SUBROUTINE TEMPORAL (NCOUNT, INTIAL, ITEM, NN)
```

```
C  
C  
C Called in MAIN to compute the fully discretized equations for the  
C parabolic and hyperbolic differential equations in time using the  
C alfa-family and Newmark family of approximations, respectively.
```

```

C
IMPLICIT REAL*8 (A-H,O-Z)
COMMON/STF/ELF(27),ELK(27,27),ELM(27,27),ELXY(9,2),ELU(27),
1           ELV(27),ELA(27),A1,A2,A3,A4,A5
C
IF (ITEM.EQ.1) THEN
C
The alfa-family of time approximation for parabolic equations
C
DO 20 I=1,NN
SUM=0.0
DO 10 J=1,NN
SUM=SUM+(ELM(I,J)-A2*ELK(I,J))*ELU(J)
10   ELK(I,J)=ELM(I,J)+A1*ELK(I,J)
20   ELF(I)=(A1+A2)*ELF(I)+SUM
ELSE
C
The Newmark integration scheme for hyperbolic equations
C
IF (NCOUNT.EQ.1 .AND. INTIAL.NE.0) THEN
  DO 40 I = 1,NN
    ELF(I) = 0.0
    DO 40 J = 1,NN
      ELF(I) = ELF(I)-ELK(I,J)*ELU(J)
      ELK(I,J)= ELM(I,J)
40 ELSE
  DO 70 I = 1,NN
    SUM = 0.0
    DO 60 J = 1,NN
      SUM = SUM+ELM(I,J)*(A3*ELU(J)+A4*ELV(J)+A5*ELA(J))
60    ELK(I,J)= ELK(I,J)+A3*ELM(I,J)
70    ELF(I) = ELF(I)+SUM
ENDIF
ENDIF
RETURN
END

```